

CHAPTER 5. STORM DRAIN SYSTEM ANALYSIS

The City's storm drain system was reviewed and analyzed with objectives to assess the system's capability to collect, convey and treat stormwater runoff, to identify problem areas, and to recommend improvements to drainage facilities that will protect property and meet the City's design standards. The review and analysis was undertaken through:

- Discussions with City staff
- Review of previous studies, reports and problem identification
- Review of plans, design drawings and as-built construction drawings
- Formulation of a dynamic model of the system based on the XP-SWMM software package
- Model analysis to identify potential problems
- Model studies to evaluate alternative projects

Drains are discussed with a description of each drain, identification of flood problems, results of the analysis, discussion of potential solutions and recommendations for improvement projects. The Master Plan focuses on the primary drainage system from the detention basin downstream including pumps and SSJID drains to the FCOC and the San Joaquin River. Included are:

- FCOC
- Drain 3 Complex including Drain 3A and Drain 3N
- Drain 4
- Drain 5 and the Drain 5 Interceptor
- Drain 7
- Drain 8
- South Drain
- SSJID Laterals

MASTER PLAN ANALYSIS

A key part of the Master Plan analysis was model studies using the XP-SWMM models developed for the City. Two basic models were developed and several other modified models were used to study specific alternative solutions to identified problems. The two models that are referenced in this Master Plan are the existing conditions model of drainage facilities in 2003 and the Master Plan model. The Master Plan model includes the General Plan land uses in the October 2003 Land Use Map that includes additional lands to be developed and changes in land uses in the previous General Plan. The Master Plan model also includes the projects identified in the plan, the operation plan for detention basins/pumps and facilities to serve new areas including Drain 3 North and the South Drain.

The Existing Conditions model was used to identify problems and evaluate solutions. The Master Plan model includes the facilities identified in the Master Plan and serves the General Plan lands.

The Master Plan analysis discussed in this chapter, including model runs, results shown on flow tables and node and flooding tables are all based on assumptions that the recommended Master Plan improvements are completed and functional. The initial analysis studied operation of detention basins and pumps with the objective of defining operation criteria that would optimize the use of detention storage in the basins to minimize downstream flooding. Basin and pump operation reflect the recommendations that resulted from these operation studies conducted with the XP-SWMM model.

SOUTH SAN JOAQUIN IRRIGATION DISTRICT REQUIREMENTS

The agreement between the City and the SSJID that governed the City's use of District drains and laterals for urban drainage expired in 1995. Drafts of new agreements have been proposed in the intervening years but at this time there is no formal agreement. Three major restrictions of the District impacted the development of the Master Plan. These are:

1. The City will not pump drainage into a SSJID drain that will exceed the capacity of the drain.
2. SSJID laterals are limited to 25 cfs of drainage inflow.
3. Stormwater entering a SSJID drain or lateral must have been treated with best management practices (BMP) in accordance with the City's NPDES permit and requirements placed on the SSJID. All discharges shall meet the requirements of the latest applicable discharge agreement with SSJID.

SSJID facilities not previously used by the City are a part of the Master Plan for use in the future including future Drain 3N flows to Drain 3, Drain 10 and Drain 11. To meet SSJID limitations, monitoring of water surface elevations and control of upstream pumps will be expanded and will be included with future pump installations.

DETENTION BASIN / PUMP OPERATION CRITERIA

Model analyses to define the existing conditions showed a number of shallow flooding areas along most drains. It was found that adjusting detention basin and pump operating criteria would alleviate much of this flooding and eliminate the violation of City standards. In the future the City XP-SWMM model can be used to optimize operation under a range of operating conditions and reduce downstream flooding problems and violations of SSJID restrictions. With the model, the City will use SSJID facilities more efficiently and remain within the District's restrictions.

A general approach to improving basin and pump operation was adopted for Master Plan analyses. The approach was to hold back on pumping from the basins allowing a greater volume of runoff to be stored. In a design storm, the objective is to fill a large percentage of the storage volume and maximize the basin's impact on downstream flows. At least one foot of freeboard was maintained in basins. Pumps were programmed to turn on when the depth in a basin reached from two to four feet below the top. It is also necessary to have a safety factor in storage if downstream monitoring and control shuts down basin pumps for a period of time.

FRENCH CAMP OUTLET CANAL

Irrigation runoff and drainage flows from the SSJID facilities and from City urban land uses terminate in the FCOC. The FCOC is owned and operated by the SSJID and the City operates under restrictions imposed by the District when the City pumps drainage runoff into drains and laterals that flow into the FCOC.

The FCOC begins at the confluence of Drains 7 and 8 just north of Highway 120. The canal follows the Union Pacific Railroad north to French Camp Slough in Stockton. Along its course north, the FCOC receives City inflow from Drain 8, Drain 7 including Laterals Y and Z, Drain 5 including Lateral Tb, Drain 4, Drain 3 including Drain 3A, Drain 3N and Laterals Re and T. Lateral Rf receives flow from Drain 3A and Pump Station 15, but in the future Drain 3A flow is expected to discharge into Drain 3N and not to the lateral. Also in the future, a portion of South Drain flow will be pumped into Drain 8 and the FCOC.

SSJID and the City sponsored an evaluation of the hydraulic capacity of the FCOC in 2001. CH2M Hill conducted the study using the Danish Hydraulic Institute (DHI) model, MIKE 11. The study evaluated the FCOC under existing conditions and with reconstruction of the several road crossings. The study found that to convey existing flows, the road crossings required improvement.

The MIKE 11 model provides a snap shot in time. Because of the complexity of the system with all the detention basins and pumps, the City elected to prepare a dynamic model, one which would take time into account and allow operation studies that would modify downstream peak flows by how basins and pumps are operated. This Master Plan update includes the dynamic model using XP-SWMM. The model was discussed in Chapter 3 and was instrumental in refining the needs for the FCOC.

The Master Plan analysis showed that reconstructing the road crossings would provide the added capacity needed to convey the Master Plan flows within the existing canal banks. The analysis showed that after reconstruction of the crossings and implementation of other upstream Master Plan improvements the FCOC would have capacity for an additional 80 cfs. The Master Plan uses this 80 cfs capacity to accommodate the flows from the first phase of the South Drain.

This Master Plan limits flow to the FCOC that can be conveyed after reconstructed road crossings but without widening the channel. If the FCOC receives flows, in addition to the Master Plan flows that include the first phase South Drain, the canal would have to be widened.

Results of Analysis

The Master Plan analysis of the FCOC included certain key assumptions. With all scenarios, there will be an increase in FCOC flows to accommodate the additional runoff. The Master Plan analysis includes all proposed FCOC road crossing improvements as having been completed. Figure 5-1 shows a schematic with model nodes labeled and Figure 5-2 shows the plan view of the FCOC.

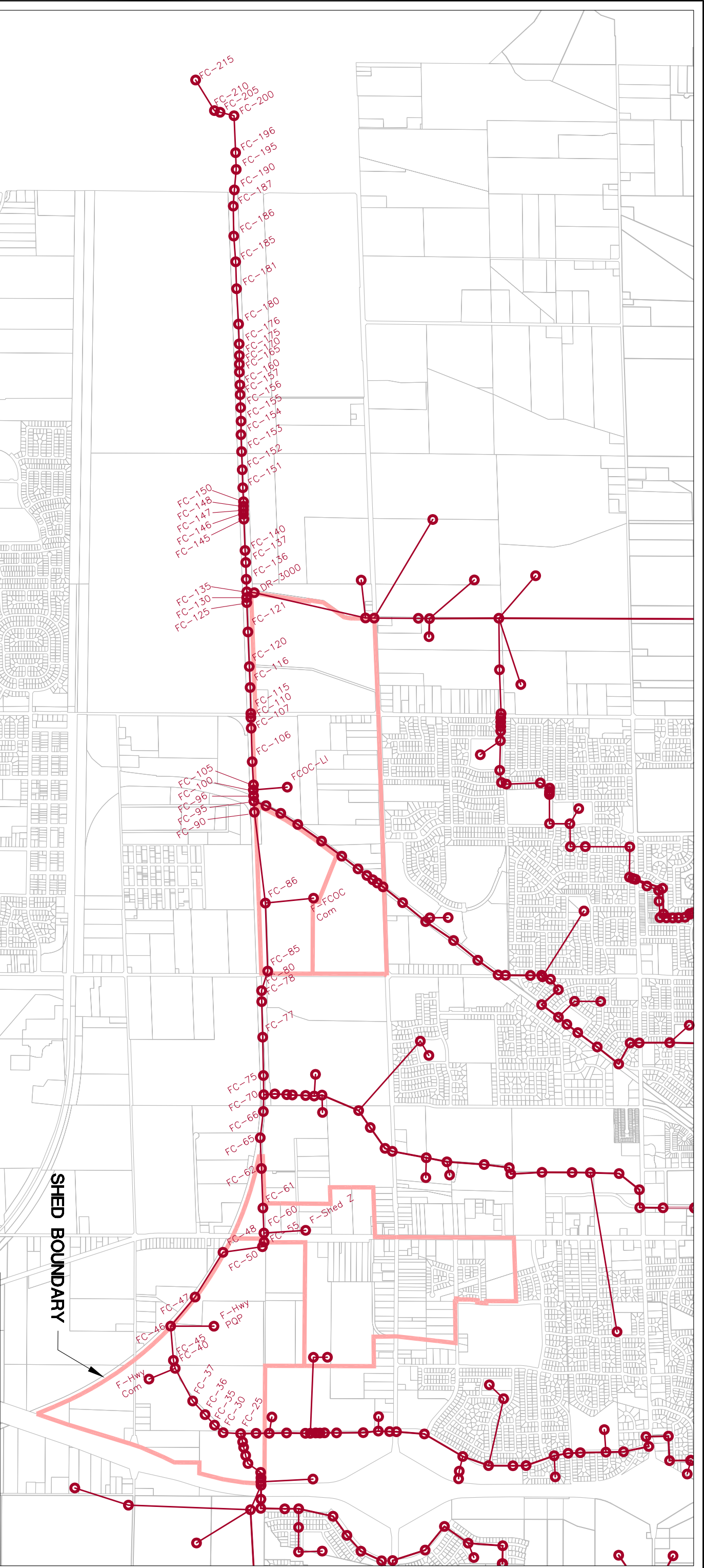


Figure 5-1

City of Manteca
Storm Drain Master Plan
FCOC SCHEMATIC



0 1000 2000
SCALE IN FEET



LEGEND

EXISTING PIPE

PROPOSED PIPE

OPEN CHANNEL

PROPOSED OPEN CHANNEL

ASSUMED LOCATION OF INFLOW FROM ADJOINING PROPERTY

ASSUMED DETENTION STORAGE

OTHER CHANNELS/PIPES

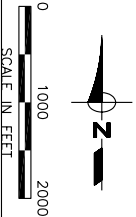


Figure 5-2

City of Manteca
Storm Drain Master Plan
FCOC DRAIN

Table 5-1, FCOC Flows, shows the peak flow rate and the maximum water surface elevation computed at each model node. Table 5-2, FCOC Water Surface Elevations, shows model output data and flooding depths at each model node. In the table, the column headed “Flood Depth” shows the water surface elevation relative to ground surface. A minus number denotes a water level below ground; a positive number (shaded) is the depth of flooding above the ground. The last column when shaded denotes a situation where City standards are violated with a water level less than six inches below ground level.

Eight individual locations show some flooding. Most are only a few inches and are still contained within the channel banks. Two locations, FC-135 and FC-137, show 0.9- and 0.7-foot potential flooding, respectively. After more detailed review, the potential flooding can be eliminated by more vigorous maintenance activities without the need for a capital project.

Master Plan Projects

The Master Plan recommended projects along the FCOC include the following replacement culverts:

Location	Recommended Improvement
• French Camp Road	Replace 66-inch culvert with two 10-foot by 10-foot box culverts
• Roth Road	Replace 42-inch and 48-inch culverts with two 10-foot by 10-foot box culverts
• Union Pacific Railroad	Replace 42-inch and 36-inch culverts with two 10-foot by 10-foot box culverts
• Louise Road	Replace 48-inch and 36-inch culverts with two 10-foot by 10-foot box culverts
• Farm Road	Replace 2-36-inch culverts with 10-foot by 8-foot box culvert

The list of FCOC road/railroad crossing culverts are summarized in Table 5-3.

DRAIN 3 AND TRIBUTARY DRAINS

Drains 3, 3A and the proposed 3N serve the northern portion of Manteca from Louise Avenue to Northland Road. Drain 3 drains 2,144 acres from Austin Road to the FCOC and has been a key facility for Manteca for many years. Drain 3A drains 385 acres north of Drain 3 from east of Highway 99 to the old Tidewater Railroad and Pump Station 15. At that point, Drain 3A flow is pumped into Lateral Rf. Drain 3N is proposed to drain 1,424 acres between Lathrop Road and Northland Road to a confluence with Drain 3 near London Road. Flows in all three drains will be combined in Drain 3 upstream of Airport Way.

Table 5-1. French Camp Outlet Canal - Flows

Upstream Node	Downstream Node	Conduit Class	Conduit Type	Max Flow, ft ³ /s	Upstream Max Water Elevation, feet	Pipe Diameter/ Box Culvert Width, feet	Box Culvert Height, feet
FC-25	FC-30	Natural	OPEN CHANNEL	191.0	21.0		
FC-30	FC-35	Rectangle	BOX CULVERT	195.8	20.9	10.0	8.0
FC-35	FC-36	Natural	OPEN CHANNEL	186.6	20.9		
FC-36	FC-37	Natural	OPEN CHANNEL	182.0	20.9		
FC-37	FC-40	Natural	OPEN CHANNEL	182.1	20.7		
FC-40	FC-45	Circular	PIPE	185.4	20.7	8.0	
FC-45	FC-46	Natural	OPEN CHANNEL	184.4	20.7		
FC-46	FC-47	Natural	OPEN CHANNEL	199.3	20.6		
FC-47	FC-48	Natural	OPEN CHANNEL	199.1	20.4		
FC-48	FC-50	Natural	OPEN CHANNEL	199.0	20.4		
FC-50	FC-55	Rectangle	BOX CULVERT	149.4	20.4	6.0	8.0
FC-50	FC-55	Rectangle	BOX CULVERT	76.3	20.4	6.0	8.0
FC-55	FC-60	Rectangle	BOX CULVERT	139.6	20.4	6.0	8.0
FC-55	FC-60	Rectangle	BOX CULVERT	85.4	20.4	6.0	8.0
FC-60	FC-61	Natural	OPEN CHANNEL	236.6	20.4		
FC-61	FC-62	Natural	OPEN CHANNEL	236.5	20.3		
FC-62	FC-65	Natural	OPEN CHANNEL	236.4	20.3		
FC-65	FC-66	Natural	OPEN CHANNEL	236.4	20.1		
FC-66	FC-70	Natural	OPEN CHANNEL	236.3	20.1		
DR-5000	FC-70	Circular	PIPE	72.0	20.0	7.0	
FC-70	FC-75	Natural	OPEN CHANNEL	277.3	20.0		
FC-75	FC-77	Natural	OPEN CHANNEL	279.7	19.8		
FC-77	FC-78	Natural	OPEN CHANNEL	277.9	19.7		
FC-78	FC-80	Natural	OPEN CHANNEL	278.4	19.7		
FC-80	FC-85	Rectangle	BOX CULVERT	139.2	19.7	10.0	10.0
FC-80	FC-85	Rectangle	BOX CULVERT	139.2	19.7	10.0	10.0
FC-85	FC-86	Natural	OPEN CHANNEL	277.2	19.7		
FC-86	FC-90	Natural	OPEN CHANNEL	282.5	19.6		
FC-90	FC-95	Rectangle	BOX CULVERT	141.5	19.5	10.0	10.0
FC-90	FC-95	Rectangle	BOX CULVERT	141.5	19.5	10.0	10.0
DR-4000	FC-95	Natural	OPEN CHANNEL	49.2	19.6		
FC-95	FC-96	Natural	OPEN CHANNEL	316.7	19.6		
FC-96	FC-100	Natural	OPEN CHANNEL	316.6	19.6		
FC-100	FC-105	Rectangle	BOX CULVERT	171.1	19.6	10.0	10.0
FC-100	FC-105	Rectangle	BOX CULVERT	171.1	19.6	10.0	10.0
FC-105	FC-106	Natural	OPEN CHANNEL	367.0	19.6		
FC-106	FC-107	Natural	OPEN CHANNEL	366.8	19.6		
FC-107	FC-110	Natural	OPEN CHANNEL	366.7	19.5		
FC-110	FC-115	Trapezoid	OPEN CHANNEL	366.8	19.5		
FC-115	FC-116	Natural	OPEN CHANNEL	367.8	19.5		
FC-116	FC-120	Natural	OPEN CHANNEL	366.7	19.5		
FC-120	FC-121	Natural	OPEN CHANNEL	419.5	19.5		
FC-121	FC-125	Natural	OPEN CHANNEL	419.4	19.4		
FC-125	FC-130	Rectangle	BOX CULVERT	209.7	19.3	10.0	10.0
FC-125	FC-130	Rectangle	BOX CULVERT	209.7	19.3	10.0	10.0
FC-130	FC-135	Natural	OPEN CHANNEL	419.4	19.2		
DR-3000	FC-135	Rectangle	BOX CULVERT	162.2	19.3	6.0	7.0
FC-135	FC-136	Natural	OPEN CHANNEL	527.5	19.2		
FC-136	FC-137	Natural	OPEN CHANNEL	527.6	19.1		
FC-137	FC-140	Natural	OPEN CHANNEL	527.6	18.9		
FC-140	FC-145	Natural	OPEN CHANNEL	537.6	18.9		
FC-145	FC-146	Natural	OPEN CHANNEL	537.6	18.8		
FC-146	FC-147	Natural	OPEN CHANNEL	537.7	18.7		
FC-147	FC-148	Natural	OPEN CHANNEL	537.7	18.6		
FC-148	FC-150	Natural	OPEN CHANNEL	537.7	18.4		
FC-150	FC-151	Natural	OPEN CHANNEL	552.7	18.2		
FC-151	FC-152	Natural	OPEN CHANNEL	552.8	18.0		
FC-152	FC-153	Natural	OPEN CHANNEL	552.8	17.9		
FC-153	FC-154	Natural	OPEN CHANNEL	552.9	17.7		
FC-154	FC-155	Natural	OPEN CHANNEL	552.9	17.6		

Table 5-1. French Camp Outlet Canal - Flows cont'd...

Upstream Node	Downstream Node	Conduit Class	Conduit Type	Max Flow, ft ³ /s	Upstream Max Water Elevation, feet	Pipe Diameter/ Box Culvert Width, feet	Box Culvert Height, feet
FC-155	FC-156	Natural	OPEN CHANNEL	568.0	17.0		
FC-156	FC-157	Natural	OPEN CHANNEL	568.0	17.0		
FC-157	FC-160	Natural	OPEN CHANNEL	568.1	16.8		
FC-160	FC-165	Rectangle	BOX CULVERT	284.1	16.6	10.0	10.0
FC-160	FC-165	Rectangle	BOX CULVERT	284.1	16.6	10.0	10.0
FC-165	FC-170	Natural	OPEN CHANNEL	568.1	16.5		
FC-170	FC-175	Rectangle	BOX CULVERT	284.1	16.5	10.0	10.0
FC-170	FC-175	Rectangle	BOX CULVERT	284.1	16.5	10.0	10.0
FC-175	FC-176	Natural	OPEN CHANNEL	568.3	16.5		
FC-176	FC-180	Natural	OPEN CHANNEL	568.5	16.2		
FC-180	FC-181	Natural	OPEN CHANNEL	593.9	16.1		
FC-181	FC-185	Natural	OPEN CHANNEL	595.0	15.6		
FC-185	FC-186	Natural	OPEN CHANNEL	621.9	15.2		
FC-186	FC-187	Natural	OPEN CHANNEL	626.2	14.3		
FC-187	FC-190	Natural	OPEN CHANNEL	631.8	13.4		
FC-190	FC-195	Rectangle	BOX CULVERT	317.0	13.2	10.0	10.0
FC-190	FC-195	Rectangle	BOX CULVERT	317.0	13.2	10.0	10.0
FC-195	FC-196	Natural	OPEN CHANNEL	729.9	13.2		
FC-196	FC-200	Natural	OPEN CHANNEL	966.5	12.6		
FC-200	FC-205	Natural	OPEN CHANNEL	644.0	12.6		
FC-205	FC-210	Trapezoid	BRIDGE CROSSING	644.0	13.1		
FC-210	FC-215	Natural	OPEN CHANNEL	644.0	12.2		

Table 5-2. French Camp Outlet Canal - Water Surface Elevations

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev
FC-30	22.9	13.9	20.9	20.8	0.1	PROBLEM
FC-35	22.9	13.15	20.9	20.8	0.1	PROBLEM
FC-36	24.3	14.3	20.9	20.6	0.3	PROBLEM
FC-37	24.8	14.4	20.7	20.3	0.4	PROBLEM
FC-40	23.3	14.86	20.7	22.3	-1.6	OK
FC-45	24.8	13.86	20.7	22.3	-1.6	OK
FC-46	24.6	14	20.6	24.1	-3.5	OK
FC-47	25.8	13.6	20.4	25.2	-4.8	OK
FC-48	28.9	13.8	20.4	26.7	-6.3	OK
FC-50	26.6	12.77	20.4	26.7	-6.3	OK
FC-55	26.6	12.12	20.4	26.7	-6.3	OK
FC-60	28.9	11.47	20.4	26.7	-6.3	OK
FC-61	28.9	13.1	20.3	24.6	-4.3	OK
FC-62	24.5	14	20.3	22.3	-2.0	OK
FC-65	24.5	14	20.1	22.3	-2.2	OK
FC-66	24.7	11.7	20.1	19.8	0.3	PROBLEM
FC-70	24.7	8.34	20.0	19.8	0.2	PROBLEM
FC-75	24.7	12.1	19.8	19.8	0.0	PROBLEM
FC-77	22.1	11.2	19.7	21.8	-2.1	OK
FC-78	21.9	10.2	19.7	21.8	-2.1	OK
FC-80	21.9	9.62	19.7	21.8	-2.1	OK
FC-85	21.9	9.53	19.7	21.8	-2.1	OK
FC-86	22.5	10.7	19.6	19.9	-0.3	OK
FC-90	22.5	10.7	19.5	19.9	-0.4	OK
FC-95	22.5	9.24	19.6	19.9	-0.3	OK
FC-96	22.5	11.6	19.6	22.0	-2.4	OK
FC-100	22.5	11.6	19.6	22.0	-2.4	OK
FC-105	22.5	11.2	19.6	22.0	-2.4	OK
FC-106	22.5	9.6	19.6	22.5	-2.9	OK
FC-107	23.4	10.4	19.5	22.7	-3.2	OK
FC-110	23.4	10.17	19.5	22.7	-3.2	OK
FC-115	23.4	9.5	19.5	22.7	-3.2	OK
FC-116	20.6	7.8	19.5	19.4	0.1	PROBLEM
FC-120	20.6	7.8	19.5	19.4	0.1	PROBLEM
FC-121	23.5	9.1	19.4	23.2	-3.8	OK
FC-125	23.5	11.7	19.3	19.6	-0.3	OK
FC-130	23.5	11.5	19.2	19.6	-0.3	OK
FC-135	18.17	10.01	19.2	18.2	1.1	PROBLEM
FC-136	20.95	10.6	19.1	19.4	-0.4	OK
FC-137	21.65	10.53	18.9	18.1	0.8	PROBLEM
FC-140	21.65	10.64	18.9	20.3	-1.4	OK
FC-145	22.1	10.49	18.8	20.1	-1.3	OK
FC-146	21.54	9.07	18.7	20.5	-1.8	OK
FC-147	23.8	9.39	18.6	23.6	-5.0	OK
FC-148	23.63	9.68	18.4	22.7	-4.2	OK
FC-150	23.24	10.57	18.2	22.2	-4.0	OK
FC-151	22.47	10.14	18.0	22.2	-4.2	OK
FC-152	22.31	10.41	17.9	20.7	-2.9	OK
FC-153	20.68	10.68	17.7	18.0	-0.4	OK
FC-154	19.56	9.44	17.6	19.6	-2.0	OK

Table 5-2. French Camp Outlet Canal - Water Surface Elevations, cont'd...

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev
FC-155	19.56	10.01	17.0	21.4	-4.4	OK
FC-156	20.98	8.51	17.0	20.5	-3.5	OK
FC-157	20.33	8.9	16.8	20.1	-3.3	OK
FC-160	23.85	9.84	16.6	23.1	-6.5	OK
FC-165	23.85	9.84	16.5	23.1	-6.6	OK
FC-170	21.92	7.1	16.5	21.6	-5.1	OK
FC-175	21.92	6.9	16.5	21.6	-5.1	OK
FC-176	24.5	6.2	16.2	22.9	-6.7	OK
FC-180	24.5	6.2	16.1	22.9	-6.8	OK
FC-181	24.3	6.1	15.6	20.3	-4.7	OK
FC-185	24.3	6.1	15.2	20.3	-5.1	OK
FC-186	24.3	6.1	14.3	21.3	-7.0	OK
FC-187	22.8	3.7	13.4	19.8	-6.4	OK
FC-190	23.7	2.2	13.2	18.7	-5.5	OK
FC-195	23	1.4	13.2	18.7	-5.5	OK
FC-196	22.8	3.7	12.6	18.8	-6.2	OK
FC-200	23	2.8	12.6	22.8	-10.2	OK
FC-205	22.8	0.5	13.1	22.8	-9.7	OK
FC-210	22.8	0.4	12.2	22.5	-10.3	OK
FC-215	23	0.3	12.2	22.8	-10.6	OK

Table 5-3. FCOC Crossing Culverts

Crossing Location	Existing Culverts	Master Plan Proposed Culverts
French Camp Road	1 - 66-inch diameter pipe culvert	2 - 10-foot by 10-foot box culvert
Roth Road	1 - 42-inch & 1 - 48-inch diameter pipe	2 - 10-foot by 10-foot box culvert
UP Intermodal	2 - 10-foot by 10-foot box culverts	—
Lathrop Road	2 - 42-inch diameter pipe culverts	—
Union Pacific Interconnect	2 - 10-foot by 8-foot box culverts	—
Former SPRR	1 - 36-inch & 1 - 42-inch pipe	2 - 10-foot by 10-foot box culvert
Louise Road	1 - 36-inch & 1 - 48-inch pipe	2 - 10-foot by 10-foot box culvert
Yosemite Road	2 - 6-foot by 8-foot box culverts	—
Farm Road	1 - 96-inch pipe culvert	—
Farm Road	2 - 36-inch diameter pipe culverts	1 - 10-foot by 8-foot box culvert

Drain 3

Drain 3 begins at a location halfway between Austin Road and Cottage Avenue east of Highway 99. The earth channel runs along the south side of Louise Avenue to a 36-inch culvert that crosses Louise Avenue and Cottage Avenue. The drain reverts to an earth channel and crosses Highway 99 in a 36-inch diameter culvert. On the west side of the highway, Drain 3 is a 48-inch diameter pipe through the Springtime Estates subdivision reverting again to an earth channel downstream. After North Main Street, the channel flows through a 48-inch pipe to the Tidewater Railroad and bike trail where it crosses the railroad in a 54-inch pipe. West of the railroad, the drain is a 60-inch diameter pipe to the Trailwood Channel where flow is carried in a 24- and 36-inch pipe. Along the channel, flow is both piped below ground and on the surface through the park in the Trailwood Channel to Union Road. West of Union Road the drain is a 60-inch pipe increasing to a 72-inch through Chadwick Square to where it crosses Lathrop Road in a 4-foot by 8-foot box culvert. North of Lathrop Road, the drain is contained in a trapezoidal earth channel to the FCOC. There is a 36-inch culvert at a small road crossing, a 36-inch culvert at Airport Way and a 7-foot by 6-foot box culvert at the FCOC.

The XP-SWMM model shows street flooding at several locations usually in the order of a few inches to a foot. The model was used to improve the operation of basins and pumps. By holding more water in each basin before turning on pumps, most of the street flooding could be averted.

The Monterey Place area has been a Drain 3 flooding problem for a long time. Flooding is caused because the drain inlets are lower than the hydraulic grade line in Drain 3. The grade at the Monterey Place drain inlet is below the top of the Drain 3 pipe. When Drain 3 is flowing full, water flows from the drain into Monterey Place from the manhole and drain inlets. There are options for protecting Monterey Place from flooding, but most would sacrifice full use of Drain 3 to protect this single area. Also, most are relatively expensive solutions.

As the City grows and Drain 3 is needed at its full capacity, it becomes more important to solve this problem without permanently reducing the drain's capacity. The best solution, although also costly, would be to disconnect the Monterey Place lateral from Drain 3, add underground storage and a pump to lift local runoff to the 60-inch drain.

Downstream of Monterey Place the Trailwood Channel winds through a small park setting. The system of a 24- and 36-inch pipe below ground and channel flow above ground appears to be working successfully with no known serious problems. Although a lower priority, the City may decide to replace this configuration with a 72-inch diameter pipe in the future.

In the open channel sections of Drain 3 downstream of Lathrop Road, there are culverts at two road crossings that are undersized and need to be replaced. The 36-inch diameter culvert at a small road crossing upstream of Airport Way and the 36-inch diameter culvert at Airport Way should both be replaced with 72-inch diameter RCP culverts. The existing 6-foot by 7-foot box culvert leading into the FCOC appears to be adequate.

Drain 3A

Drain 3A drains 287 acres east of Highway 99 crossing to the west under the highway and Main Street in a 30-inch diameter pipe. The drain continues west in a 36-inch pipe through Crestwood Park to the Tidewater Railroad where it turns north crossing Lathrop Road. The present drain flows north of Lathrop Road for approximately 1,500 feet to Pump Station 15 where flow is pumped into Lateral Rf.

Runoff in the Drain 3A shed from east of Highway 99 will be held in the two detention basins to be constructed until the water level in the 36-inch pipe at Main Street has lowered indicating that the downstream pipe can carry the additional flow. Operation of the pumps at the two proposed basins will be controlled remotely from the Main Street water surface elevation.

Flow in Drain 3A is controlled by its 36-inch diameter. The Master Plan analysis did not show flooding problems along the drain but no increase in capacity is possible without replacing the pipe with a pipe larger than 36 inches. It is proposed to connect Drain 3A to the proposed Drain 3N at Lathrop Road. Flow line and HGL elevations for the two pipes show that it may be possible for Drain 3A to flow into Drain 3N without the need for a pump. The planned invert of the Drain 3N pipe is 20.1 feet and the Drain 3A invert is at 22.3 feet.

Drain 3N

A proposed new drain, Drain 3N, is planned to serve new growth areas of the General Plan. The drain would serve the most northern area of the City, primarily the land between Lathrop Road and Northland Road. West of Union Road the new drain would join with Drain 3 and in the east, the drain would extend to between Cottage Avenue and Austin Road. Drain 3N would drain about 1,424 acres bringing a total flow of 55cfs to its confluence with Drain 3.

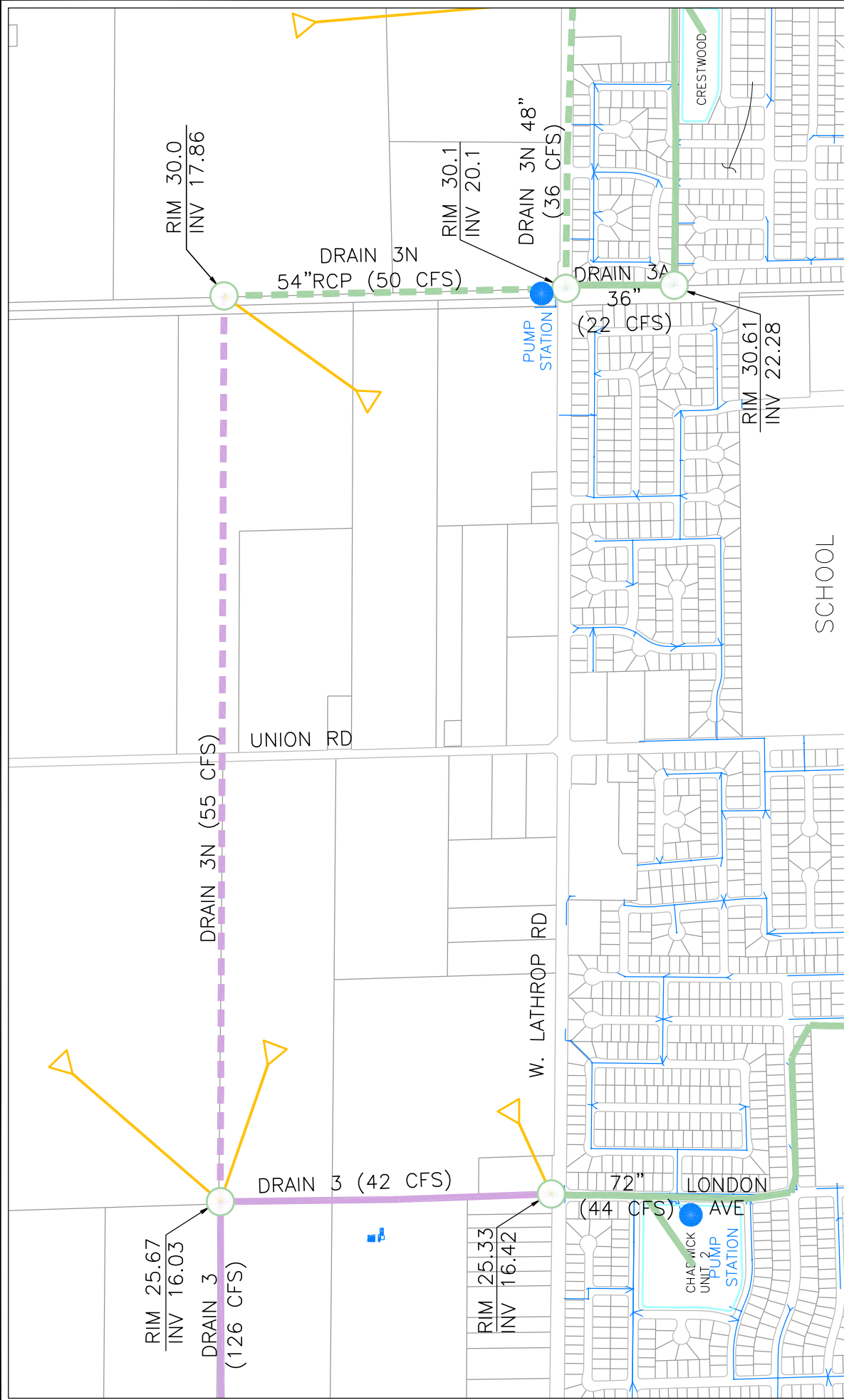
Lands to 2,640 feet north of Lathrop Road are planned for development during the next several years. General Plan lands further north are in urban reserve and are not included in the present Master Plan. The preliminary alignment for Drain 3N along Lathrop Road shown in previous Master Plan updates is maintained in the present plan because the location works well east of Highway 99 where land north and south of Lathrop Road will drain to Drain 3N. After crossing the highway, the new drain would continue along Lathrop Road until the Tidewater Railroad along the west boundary of the San Joaquin Delta College Farm Laboratory where the drain will turn north.

Drain 3A flow enters Drain 3N at the point where Drain 3N turns from Lathrop Road to the north. The combined Drain 3N/Drain 3A would flow north in a 54 inch diameter pipe for approximately 2,100 feet where the drain will turn to the west and continue for 2,100 feet in an open drainage channel to its confluence with Drain 3. Drain 3N will follow the Tidewater Bikeway alignment from Lathrop Road to its confluence with Drain 3, Figure 5-3.

Results of Analysis

Figure 5-4 shows the model schematic of Drains 3, 3A and 3N with model nodes labeled. Table 5-4, Drain 3 Flows, shows the peak flow rate and the maximum water surface elevation computed at each model node. Table 5-5, Drain 3 Water Surface Elevations, shows model output data and flooding depths at each model node with a problem designation when flooding is computed above ground level. A plan of Drain 3, Drain 3N and Drain 3A is shown on Figure 5-5.

Monterey Place is flood prone largely because the Monterey Place drain and drain inlets are below the hydraulic grade line in Drain 3. Several of the solutions considered by the City involved under utilizing the capacity of Drain 3 by limiting flow or installing larger drain pipes for hundreds of feet. Drain 3 should not be compromised by one small street drainage. The recommendation for Monterey Place is to divert the local drains to a constructed sump with a volume of 0.65 acre-feet and after detention pump to Drain 3 at a rate of 73 gpm.



LEGEND

- EXISTING PIPE
- PROPOSED PIPE
- OPEN CHANNEL
- PROPOSED OPEN CHANNEL
- ASSUMED LOCATION OF INFLOW
- FROM ADJOINING PROPERTY
- ASSUMED DETENTION STORAGE
- OTHER CHANNELS/PIPES

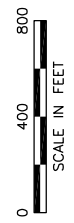


Figure 5-3

City of Manteca
Storm Drain Master Plan
DRAIN 3 NORTH



Figure 5-4

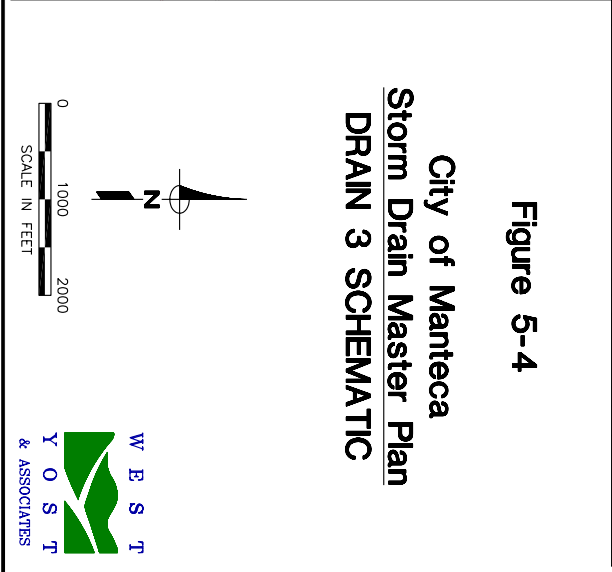
City of Manteca
Storm Drain Master Plan
DRAIN 3 SCHEMATIC

The schematic map shows the layout of Drain 3, including its main channel and various tributaries. Key features include the Manteca River, Manteca Creek, and several smaller creeks and drains. The map is oriented with North at the top. A scale bar indicates distances up to 2000 feet. The legend identifies the project as 'W E S T Y O S T & ASSOCIATES'.

Figure 5-4

City of Manteca
Storm Drain Master Plan
DRAIN 3 SCHEMATIC

The schematic map shows the layout of Drain 3, including its main channel and various tributaries. Key features include the Manteca River, Manteca Creek, and several smaller creeks and drains. The map is oriented with North at the top. A scale bar indicates distances up to 2000 feet. The legend identifies the map as 'W E S T Y O S T & ASSOCIATES'.



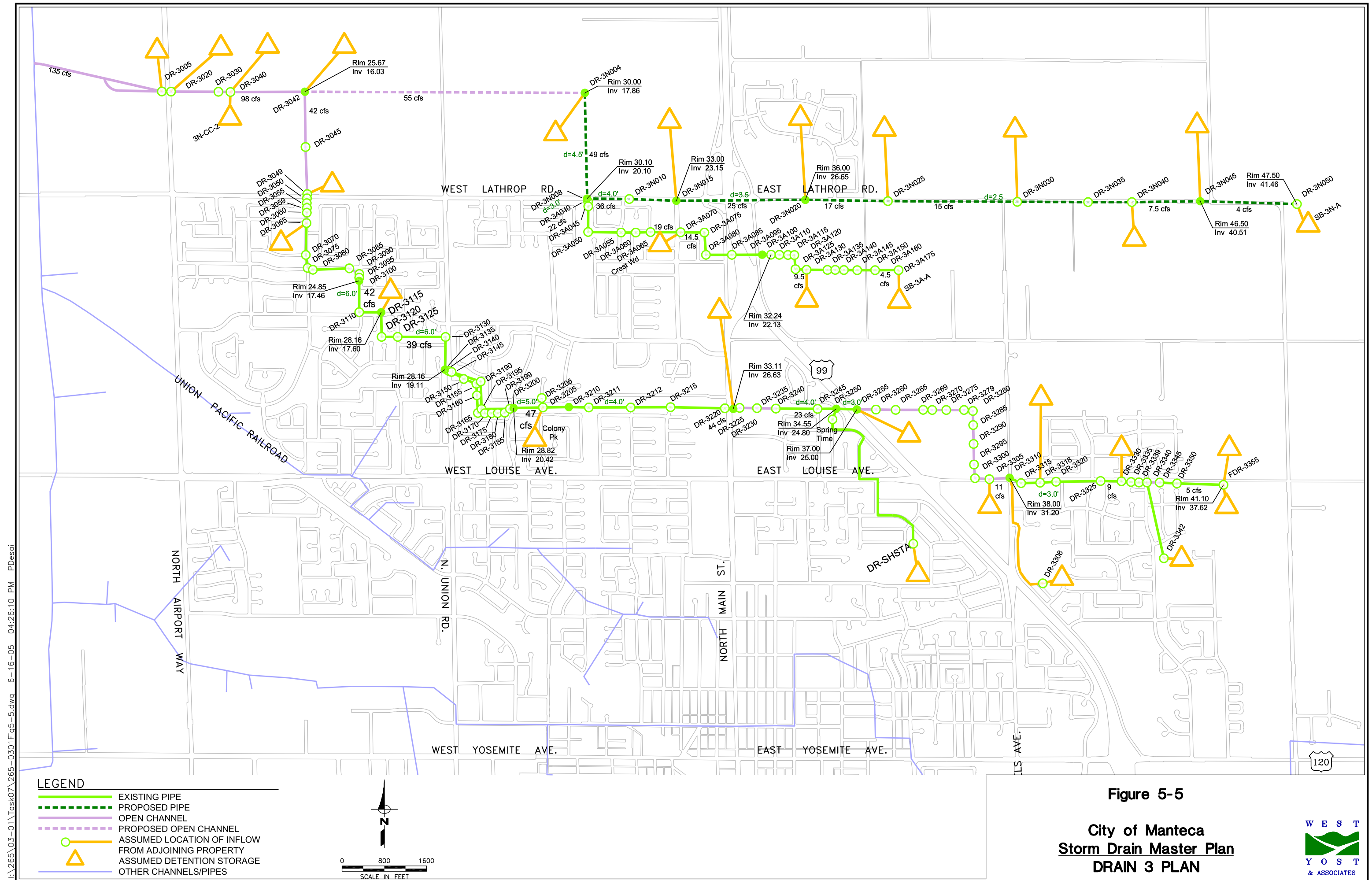


Table 5-4. Drain 3 Complex - Flows

Conduit Name	Upstream Node	Downstream Node	Conduit Class	Conduit Type	Max Flow, ft ³ /s	Upstream Max Water Elevation, feet	Pipe Diameter/ Box Culvert Width, feet	Box Culvert Height, feet
FDRP3350	FDR-3355	DR-3350	Circular	PIPE	5.1	38.6	3.0	
DRC3345	DR-3350	DR-3345	Natural	OPEN CHANNEL	5.3	36.2		
DRC3340	DR-3345	DR-3340	Natural	OPEN CHANNEL	5.3	35.0		
DRP3340A	DR-3342	DR-3340	Circular	PIPE	6.1	37.0	2.0	
DRP3339	DR-3340	DR-3339	Circular	PIPE	7.5	34.1	3.0	
DRC3335	DR-3339	DR-3335	Natural	OPEN CHANNEL	7.1	34.1		
DRC3330	DR-3335	DR-3330	Natural	OPEN CHANNEL	6.9	34.0		
DRP3325	DR-3330	DR-3325	Circular	PIPE	9.0	34.0	3.0	
DRP3320	DR-3325	DR-3320	Circular	PIPE	9.0	34.0	3.0	
DRP3318	DR-3320	DR-3318	Circular	PIPE	9.0	33.8	3.0	
DRP3315.1	DR-3318	DR-3315	Circular	PIPE	12.4	33.7	3.0	
DRP3310	DR-3315	DR-3310	Circular	PIPE	12.4	33.6	3.0	
DRP3310A	DR-3308	DR-3310	Circular	PIPE	1.6	37.3	1.3	
DRC3305	DR-3310	DR-3305	Natural	OPEN CHANNEL	13.8	32.6		
DRC3300	DR-3305	DR-3300	Natural	OPEN CHANNEL	14.5	32.6		
DRC3295	DR-3300	DR-3295	Natural	OPEN CHANNEL	14.1	32.4		
DRC3290	DR-3295	DR-3290	Natural	OPEN CHANNEL	13.7	32.3		
DRC3285	DR-3290	DR-3285	Natural	OPEN CHANNEL	13.4	32.2		
DRC3280	DR-3285	DR-3280	Natural	OPEN CHANNEL	13.1	32.1		
DRP3279	DR-3280	DR-3279	Circular	PIPE	13.1	32.1	3.0	
DRC3275	DR-3279	DR-3275	Natural	OPEN CHANNEL	12.9	32.0		
DRC3270	DR-3275	DR-3270	Natural	OPEN CHANNEL	12.8	31.9		
DRP3269	DR-3270	DR-3269	Circular	PIPE	12.8	31.9	3.0	
DRC3265	DR-3269	DR-3265	Natural	OPEN CHANNEL	12.7	31.8		
DRC3260	DR-3265	DR-3260	Natural	OPEN CHANNEL	12.7	31.6		
DRC3255	DR-3260	DR-3255	Natural	OPEN CHANNEL	12.7	31.3		
DRP3250	DR-3255	DR-3250	Circular	PIPE	12.6	31.3	3.0	
DRPSPRTM	DR-SHSTA	SPRINGTIME	Circular	PIPE	4.2	33.9	1.8	
DRP3250A	SPRINGTIME	DR-3250	Circular	PIPE	19.5	31.3	2.0	
DRP3245	DR-3250	DR-3245	Circular	PIPE	22.8	31.2	4.0	
DRP3240	DR-3245	DR-3240	Circular	PIPE	24.6	31.2	4.0	
DRC3235	DR-3240	DR-3235	Natural	OPEN CHANNEL	25.0	31.2		
DRC3230	DR-3235	DR-3230	Natural	OPEN CHANNEL	26.0	31.2		
DRC3225	DR-3225	DR-3230	Natural	OPEN CHANNEL	36.2	31.2		
DRP3225A	KMart	DR-3225	Circular	PIPE	3.9	38.2	1.3	
DRP3220	DR-3225	DR-3220	Circular	PIPE	44.7	31.2	4.0	
DRP3215	DR-3220	DR-3215	Circular	PIPE	44.6	31.0	4.0	
DRP3212	DR-3215	DR-3212	Circular	PIPE	46.8	29.6	4.0	
DRP3211	DR-3212	DR-3211	Circular	PIPE	49.7	28.6	4.0	
DRP3210	DR-3211	DR-3210	Circular	PIPE	54.1	27.5	4.0	
DRP3205	DR-3210	DR-3205	Circular	PIPE	59.1	26.8	5.0	
DRP3205A	ColonyPk	DR-3205	Circular	PIPE	11.4	25.7	2.0	
DRP3205B.1	DR-3206	DR-3205	Circular	PIPE	1.2	26.5	1.3	
DRP3200	DR-3205	DR-3200	Circular	PIPE	48.6	26.4	5.0	
DRP3199	DR-3200	DR-3199	Circular	PIPE	52.3	26.3	5.0	
DRP3185	DR-3199	DR-3185	Circular	PIPE	22.7	26.2	2.0	
DRP3195	DR-3199	DR-3195	Circular	PIPE	29.3	26.2	3.0	
DRP3190	DR-3195	DR-3190	Circular	PIPE	29.3	25.0	3.0	
DRP3145B	DR-3190	DR-3145	Circular	PIPE	29.3	23.8	3.0	
DRP3180	DR-3185	DR-3180	Circular	PIPE	22.7	25.6	2.0	
DRP3175	DR-3180	DR-3175	Circular	PIPE	11.3	24.6	2.0	
DRP3170	DR-3175	DR-3170	Circular	PIPE	9.8	24.3	2.0	
DRP3165	DR-3170	DR-3165	Circular	PIPE	8.4	24.1	2.0	
DRP3160	DR-3165	DR-3160	Circular	PIPE	8.4	23.8	2.0	
DRP3155	DR-3160	DR-3155	Circular	PIPE	8.4	23.3	2.0	
DRP3150	DR-3155	DR-3150	Circular	PIPE	8.4	22.9	2.0	
DRP3145A	DR-3150	DR-3145	Circular	PIPE	8.4	22.6	2.0	
DRP3140	DR-3145	DR-3140	Circular	PIPE	36.0	22.2	5.0	
DRP3135	DR-3140	DR-3135	Circular	PIPE	36.0	22.2	5.0	
DRP3130	DR-3135	DR-3130	Circular	PIPE	43.0	22.2	5.0	

Table 5-4. Drain 3 Complex - Flows, cont'd...

	Upstream Node	Downstream Node	Conduit Class	Conduit Type	Max Flow, ft ³ /s	Upstream Max Water Elevation, feet	Pipe Diameter/ Box Culvert Width, feet	Box Culvert Height, feet
DRP3125	DR-3130	DR-3125	Circular	PIPE	42.7	21.9	5.0	
DRP3120	DR-3125	DR-3120	Circular	PIPE	42.4	21.5	5.0	
DRP3115	DR-3120	DR-3115	Circular	PIPE	42.3	21.3	5.0	
DRP3110	DR-3115	DR-3110	Circular	PIPE	45.8	21.2	6.0	
DRP3100	DR-3110	DR-3100	Circular	PIPE	45.6	21.2	6.0	
DRP3095	DR-3095	DR-3100	Circular	PIPE	45.5	21.0	6.0	
DRP3090	DR-3095	DR-3090	Circular	PIPE	45.4	21.0	6.0	
DRP3085	DR-3090	DR-3085	Circular	PIPE	45.3	20.9	6.0	
DRP3080	DR-3085	DR-3080	Circular	PIPE	45.2	20.8	6.0	
DRP3075	DR-3080	DR-3075	Circular	PIPE	45.1	20.8	6.0	
DRP3070	DR-3075	DR-3070	Circular	PIPE	45.0	20.7	6.0	
DRP3065	DR-3070	DR-3065	Circular	PIPE	44.9	20.7	6.0	
DRP3060	DR-3065	DR-3060	Circular	PIPE	44.8	20.5	6.0	
DRP3059	DR-3060	DR-3059	Circular	PIPE	44.7	20.4	6.0	
DRP3055	DR-3059	DR-3055	Circular	PIPE	44.6	20.4	6.0	
DRP3050	DR-3055	DR-3050	Circular	PIPE	44.5	20.3	6.0	
DRBX3049	DR-3050	DR-3049	Rectangle	BOX CULVERT	44.5	20.3	8.0	4.0
DRC3040	DR-3049	DR-3045	Natural	OPEN CHANNEL	43.8	20.3		
DRC3042	DR-3045	DR-3042	Natural	OPEN CHANNEL	41.4	20.3		
DRC160/70	DR-3A175	DR-3A160	Natural	OPEN CHANNEL	4.7	34.3		
DRC3A150	DR-3A160	DR-3A150	Natural	OPEN CHANNEL	3.7	32.8		
DRP3A145	DR-3A150	DR-3A145	Circular	PIPE	3.7	32.7	2.5	
DRC3A140	DR-3A145	DR-3A140	Natural	OPEN CHANNEL	3.7	32.3		
DRC3A135	DR-3A140	DR-3A135	Natural	OPEN CHANNEL	3.7	31.9		
DRC3A130	DR-3A135	DR-3A130	Natural	OPEN CHANNEL	3.7	31.4		
DRC3A125	DR-3A130	DR-3A125	Natural	OPEN CHANNEL	9.5	30.4		
DRC3A120	DR-3A125	DR-3A120	Natural	OPEN CHANNEL	9.5	29.9		
DRP3A115	DR-3A120	DR-3A115	Circular	PIPE	9.5	28.7	2.0	
DRC3A110	DR-3A115	DR-3A110	Natural	OPEN CHANNEL	9.5	28.5		
DRC3A100/5	DR-3A110	DR-3A100	Natural	OPEN CHANNEL	9.5	28.5		
DRC3A095	DR-3A100	DR-3A095	Natural	OPEN CHANNEL	9.6	28.5		
DRP3A085	DR-3A095	DR-3A085	Circular	PIPE	9.5	28.5	3.0	
DRP3A080	DR-3A085	DR-3A080	Circular	PIPE	16.1	28.4	3.0	
DRP3A075	DR-3A080	DR-3A075	Circular	PIPE	14.5	28.1	3.0	
DRP3A070	DR-3A075	DR-3A070	Circular	PIPE	14.5	27.9	3.0	
DRP3A070A	Crest Wd	DR-3A070	Circular	PIPE	6.9	27.7	2.5	
DRP3A065	DR-3A070	DR-3A065	Circular	PIPE	18.6	27.6	3.0	
DRP3A060	DR-3A065	DR-3A060	Circular	PIPE	18.6	27.1	3.0	
DRP3A055	DR-3A060	DR-3A055	Circular	PIPE	22.1	26.8	3.0	
DRP3A050	DR-3A055	DR-3A050	Circular	PIPE	22.1	26.4	3.0	
DRP3A045	DR-3A050	DR-3A045	Circular	PIPE	22.1	25.5	3.0	
DRP3A040	DR-3A045	DR-3A040	Circular	PIPE	22.1	24.4	3.0	
DRP3N045	DR-3N050	DR-3N045	Circular	PIPE	3.9	42.8	2.0	
DRP3N040	DR-3N045	DR-3N040	Circular	PIPE	7.4	42.2	2.5	
DRP3N035	DR-3N040	DR-3N035	Circular	PIPE	11.3	41.7	2.5	
DRP3N030	DR-3N035	DR-3N030	Circular	PIPE	11.3	40.4	2.5	
DRP3N025	DR-3N030	DR-3N025	Circular	PIPE	14.9	38.9	2.5	
DRP3N020	DR-3N025	DR-3N020	Circular	PIPE	16.7	32.5	2.5	
DRP3N015	DR-3N020	DR-3N015	Circular	PIPE	24.6	28.9	3.5	
DRP3N010	DR-3N015	DR-3N010	Circular	PIPE	35.5	25.8	4.0	
DRP3N008	DR-3N010	DR-3N008	Circular	PIPE	35.6	24.2	4.0	
DRP3N008A	DR-3A040	DR-3N008	Circular	PIPE	22.1	24.0	3.0	
DRP3N004	DR-3N008	DR-3N004	Circular	PIPE	49.1	23.4	4.5	
DRC3N042	DR-3N004	DR-3042	Trapezoid	OPEN CHANNEL	54.9	20.6		
DRC3040A	DR-3042	DR-3040	Natural	OPEN CHANNEL	125.5	20.2		
DRP3030	DR-3040	DR-3030	Rectangle	BOX CULVERT	128.1	19.8	7.0	6.0
DRC3020	DR-3030	DR-3020	Natural	OPEN CHANNEL	127.9	19.8		
DRBX3005	DR-3020	DR-3005	Rectangle	BOX CULVERT	127.8	19.7	7.0	6.0
DRC3000	DR-3005	DR-3000	Natural	OPEN CHANNEL	163.2	19.7		

Table 5-5. Drain 3 Complex - Water Surface Elevations

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev	Notes
DR-3350	40.1	35.38	36.2	39.9	-3.6	OK	
DR-3345	39.9	34.5	35.0	39.6	-4.6	OK	
SprgMeadw	42	19.2	35.8	41.7	-5.9	OK	
DR-3342	41.2	36.62	37.0	40.9	-3.9	OK	
DR-3340	39.43	32.07	34.1	39.1	-5.0	OK	
DR-3339	39.43	32	34.1	39.1	-5.0	OK	
DR-3335	38.9	33.15	34.0	38.9	-4.9	OK	
DiamdOaks	40	26	35.9	39.7	-3.8	OK	
DR-3330	37.75	30.93	34.0	37.5	-3.4	OK	
DR-3325	38.52	30.57	34.0	38.2	-4.2	OK	
DR-3320	39.04	29.74	33.8	38.7	-4.9	OK	
SB-3-D	39	29.096	31.2	38.7	-7.5	OK	
DR-3318	39	29.096	33.7	38.7	-5.0	OK	
DR-3315	38.95	28.65	33.6	38.7	-5.0	OK	
ButtonEst	41.08	23.32	35.9	40.8	-4.9	OK	
DR-3308	41.6	37.09	37.3	41.3	-4.0	OK	
DR-3310	38	31.2	32.6	37.7	-5.1	OK	
DR-3305	37.2	30.8	32.6	37.2	-4.6	OK	
DR-3300	36.9	30.5	32.4	36.6	-4.2	OK	
DR-3295	38.45	29.7	32.3	36.9	-4.6	OK	
DR-3290	39.2	30.1	32.2	37.8	-5.5	OK	
DR-3285	38.05	30.2	32.1	37.5	-5.3	OK	
DR-3280	40	29.84	32.1	39.7	-7.6	OK	
DR-3279	40	29.76	32.0	39.7	-7.7	OK	
DR-3275	40.05	29.7	31.9	39.2	-7.2	OK	
DR-3270	39.5	30.1	31.9	39.2	-7.3	OK	
DR-3269	39.5	30.03	31.8	39.2	-7.4	OK	
DR-3265	40.55	30.15	31.6	40.1	-8.5	OK	
DR-3260	40.3	29.45	31.3	40.1	-8.8	OK	
DR-3255	37	25	31.3	36.7	-5.4	OK	
SHASTAPRK	39	21	35.9	38.7	-2.8	OK	
DR-SHSTA	38	30.41	33.9	37.7	-3.8	OK	
SPRINGTIME	33	26	31.3	32.7	-1.4	OK	
DR-3250	34.55	24.8	31.2	34.3	-3.0	OK	
DR-3245	33.27	24.62	31.2	33.0	-1.8	OK	
DR-3240	34.5	24.98	31.2	34.2	-3.0	OK	
DR-3235	35.9	25.6	31.2	33.7	-2.5	OK	
DR-3230	35.6	25.5	31.2	33.4	-2.1	OK	
KMart	40.31	36.23	38.2	40.0	-1.8	OK	
DR-3225	33.11	26.63	31.2	32.8	-1.6	OK	
DR-3220	32.81	24.92	31.0	32.5	-1.5	OK	
DR-3215	30.72	24.67	29.6	30.4	-0.9	OK	
DR-3212	30.22	24.32	28.6	29.9	-1.3	OK	
DR-3211	29.7	23.96	27.5	29.4	-1.9	OK	
DR-3210	29.49	23.8	26.8	29.2	-2.4	OK	
ColonyPk	30	23.048	25.7	29.7	-4.0	OK	
DR-3206	30	24	26.5	29.7	-3.2	OK	
DR-3205	29.5	22.06	26.5	29.2	-2.8	OK	
DR-3200	28.82	20.42	26.3	28.5	-2.3	OK	
DR-3199	28.42	20.5	26.2	28.1	-1.9	OK	

Table 5-5. Drain 3 Complex - Water Surface Elevations cont'd...

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev
DR-3195	25.67	20.67	25.0	25.4	-0.4	OK
DR-3190	25.98	20.08	23.8	25.7	-1.8	OK
DR-3185	28.02	21.2	25.6	27.7	-2.1	OK
DR-3180	24.03	21.13	24.6	23.7	0.8	PROBLEM
DR-3175	23.95	21.05	24.3	23.7	0.6	PROBLEM
DR-3170	23.63	20.93	24.1	23.3	0.7	PROBLEM
DR-3165	23.85	20.85	23.8	23.6	0.2	PROBLEM
DR-3160	23.51	20.46	23.3	23.2	0.1	PROBLEM
DR-3155	23.34	20.24	22.9	23.0	-0.1	PROBLEM
DR-3150	27.18	20	22.6	26.9	-4.3	OK
DR-3145	26.56	19.66	22.2	26.3	-4.0	OK
DR-3140	28.16	19.11	22.2	27.9	-5.6	OK
DR-3135	27	18.9	22.2	26.7	-4.5	OK
DR-3130	27	18.5	21.9	26.7	-4.8	OK
DR-3125	26.8	17.9	21.5	26.5	-5.0	OK
DR-3120	28.15	17.68	21.3	27.9	-6.5	OK
Doxey	26	8.5	23.5	25.7	-2.2	OK
DR-3115	27.28	17.6	21.2	27.0	-5.7	OK
DR-3110	25.62	17.79	21.2	25.3	-4.2	OK
DR-3100	24.85	17.46	21.0	24.6	-3.5	OK
DR-3095	27	17.33	21.0	25.3	-4.2	OK
DR-3090	26.8	17.23	20.9	26.6	-5.6	OK
DR-3085	26.65	17.08	20.8	26.7	-5.8	OK
DR-3080	27.2	17.07	20.8	26.4	-5.6	OK
DR-3075	26.7	16.99	20.7	26.3	-5.5	OK
DR-3070	26.2	16.94	20.7	25.9	-5.2	OK
Chdwk	23.5	7.04	19.1	23.2	-4.1	OK
DR-3065	25.9	16.92	20.5	25.7	-5.2	OK
DR-3060	26.4	16.83	20.4	25.7	-5.3	OK
DR-3059	26	16.8	20.4	25.7	-5.3	OK
DR-3055	26.85	16.77	20.3	26.3	-5.9	OK
SB-3N-H	30	20	23.0	29.7	-6.7	OK
SB-3N-A	47.5	40	41.0	47.2	-6.2	OK
DR-3N050	47.5	41.46	42.8	47.2	-4.4	OK
SB-3N-B	46.5	39	40.0	46.2	-6.2	OK
DR-3N045	46.5	40.51	42.2	46.2	-4.0	OK
SB-3N-C	46	38.5	39.5	45.7	-6.2	OK
DR-3N040	46	39.95	41.7	45.7	-4.0	OK
DR-3N035	45	38.6	40.4	44.7	-4.3	OK
SB-3N-D	46	38	39.0	45.7	-6.7	OK
DR-3N030	46	37.21	38.9	45.7	-6.8	OK
SB-3N-E	40	31.5	32.5	39.7	-7.2	OK
DR-3N025	40	30.71	32.5	39.7	-7.2	OK
SB-3N-F	36	27.5	29.5	35.7	-6.2	OK
DR-3N020	36	26.65	28.9	35.7	-6.8	OK
SB-3N-G	33	24	26.4	32.7	-6.3	OK
DR-3N015	33	23.15	25.8	32.7	-6.9	OK
DR-3N010	30.2	20.94	24.2	29.9	-5.7	OK
SB-3A-A	40.8	33.61	34.9	40.5	-5.6	OK
DR-3A175	40.8	33.61	34.3	37.6	-3.3	OK
DR-3A160	39.4	29.94	32.8	39.1	-6.3	OK

Table 5-5. Drain 3 Complex - Water Surface Elevations cont'd...

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev	
DR-3A150	38.94	31.91	32.7	38.6	-5.9	OK	
DR-3A145	38.94	31.89	32.3	38.6	-6.3	OK	
DR-3A140	39.15	31.73	31.9	37.3	-5.4	OK	
DR-3A135	37.48	30.8	31.4	36.5	-5.0	OK	
SB-3A-B	36.65	29.52	31.3	36.4	-5.1	OK	
DR-3A130	36.65	29.52	30.4	36.3	-5.9	OK	
DR-3A125	36.57	29.35	29.9	35.9	-6.0	OK	
DR-3A120	36	26.65	28.7	35.7	-7.0	OK	
DR-3A115	36	26.85	28.5	35.7	-7.2	OK	
DR-3A110	35.29	27.79	28.5	35.2	-6.7	OK	
DR-3A100	34.01	26.51	28.5	33.6	-5.1	OK	
DR-3A095	32.24	22.13	28.5	31.9	-3.5	OK	
DR-3A085	32.15	26.16	28.4	31.9	-3.5	OK	
DR-3A080	32.36	24.63	28.1	32.1	-3.9	OK	
DR-3A075	33.34	24.55	27.9	33.0	-5.1	OK	
Crest Wd	32.13	26.96	27.7	31.8	-4.1	OK	
DR-3A070	32.42	20.95	27.6	32.1	-4.5	OK	
DR-3A065	31	22.31	27.1	30.7	-3.6	OK	
DR-3A060	31.04	22.26	26.8	30.7	-3.9	OK	
DR-3A055	30.67	21.99	26.4	30.4	-4.0	OK	
DR-3A050	32.04	21.67	25.5	31.7	-6.3	OK	
DR-3A045	32.05	22.31	24.4	31.8	-7.4	OK	
DR-3A040	30.61	22.28	24.0	30.3	-6.4	OK	
DR-3N008	30.1	20.1	23.4	29.8	-6.4	OK	
DR-3N004	30	17.86	20.6	29.7	-9.1	OK	
DR-3050	25.33	16.74	20.3	25.0	-4.7	OK	
DR-3049	25.33	16.72	20.3	25.0	-4.7	OK	
DR-3045	25.33	16.27	20.3	25.0	-4.7	OK	
SB-F-LDRES	25.67	16.03	17.6	25.4	-7.8	OK	
3N-CC-1	25.33	16.42	18.3	25.0	-6.7	OK	
3N-CC-2	26	15.79	17.9	25.7	-7.8	OK	
DR-3042	25.67	16.03	20.2	25.4	-5.1	OK	
DR-3040	26	15.79	19.8	25.7	-5.9	OK	
X-DR3-B	26	15.54	17.5	25.7	-8.2	OK	
DR-3030	26	15.54	19.8	25.7	-5.9	OK	
DR-3020	26	13.5	19.7	25.7	-6.0	OK	
X-DR3-A	23	13.42	16.8	22.7	-5.9	OK	
DR-3005	23	13.42	19.7	22.7	-3.1	OK	
DR-3000	20	10.5	19.3	19.7	-0.4	OK	
SB-3-A	40.1	37.62	38.2	39.8	-1.6	OK	
SB-3-B	40.1	37.62	38.2	39.8	-1.6	OK	
SB-3-D	39	29.096	31.2	38.7	-7.5	OK	
SB-3-E	37.2	30.8	36.5	36.9	-0.4	OK	

A detailed feasibility study is required to compare alternatives and select a preferred solution for Monterey Place flooding. Each potential solution has positives and negatives that require more detailed analysis than is possible within this master plan.

Six locations in Table 5-5 show some flooding from 0.3 to 0.9 foot deep. The flooding area is actually the Trailhead Channel and there is no flood problem because the area was designed to carry overland releases.

Master Plan Projects

- | | |
|--------------------------------------------------|---------------------------------------------------------|
| • Drain 3N, Tidewater RR to Drain 3 | 5,300 feet of Drain 3N Channel |
| • Drain 3N, Lathrop Road north | 2,100 ft of Drain 3N 54-inch pipe |
| • Drain 3A, Lathrop Road | Connect 3A to 3N |
| • Drain 3, Monterey Place | Pipe modifications, sump and pump connection to Drain 3 |
| • Drain 3, Airport Way | 6-foot by 8-foot box culvert |
| • Drain 3, road crossing upstream of Airport Way | 6-foot by 8-foot box culvert |

DRAIN 4

Drain 4 drains 885 acres of central Manteca to the French Camp Outlet Canal. The drain begins west of Main Street and flows in a 36 inch diameter pipe generally west to Graystone Park where it increases to a 48-inch pipe. The drain flows northwest along the Union Pacific Railroad past Mayors Park to Louise Avenue where the pipe is increased to 54 inches in diameter. The 54-inch pipe continues west to the railroad tracks and turns northwest and flows through a 60-inch culvert at Airport Way. An earth channel carries flow from Airport Way to the French Camp Outlet Canal.

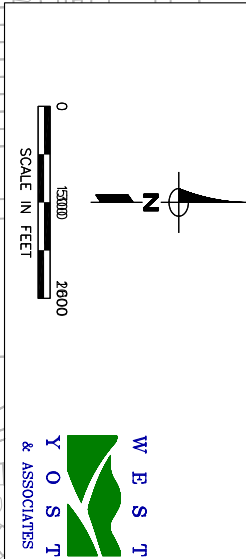
Analysis in previous Master Plans and for this Master Plan show that Drain 4 is close to capacity. Minor street flooding is evident at several locations. Although not meeting City standards, most of the flooding seen in the model is more of a nuisance and probably not worth large expenditures to replace existing pipes with larger pipes. Each of the drains benefits from the road crossing improvements along the FCOC. Master Plan analysis includes completion of the FCOC improvements.

West of Airport Way, Drain 4 is an earth channel through land primarily designated for both light and heavy industrial uses. On the north side of the drain 102 acre feet of storage will be required and on the south side 21 acre feet is required. As lands west of Airport Way develop, the City may elect to use an open drainage corridor or drain 4 may be converted to a 54-inch diameter pipe.

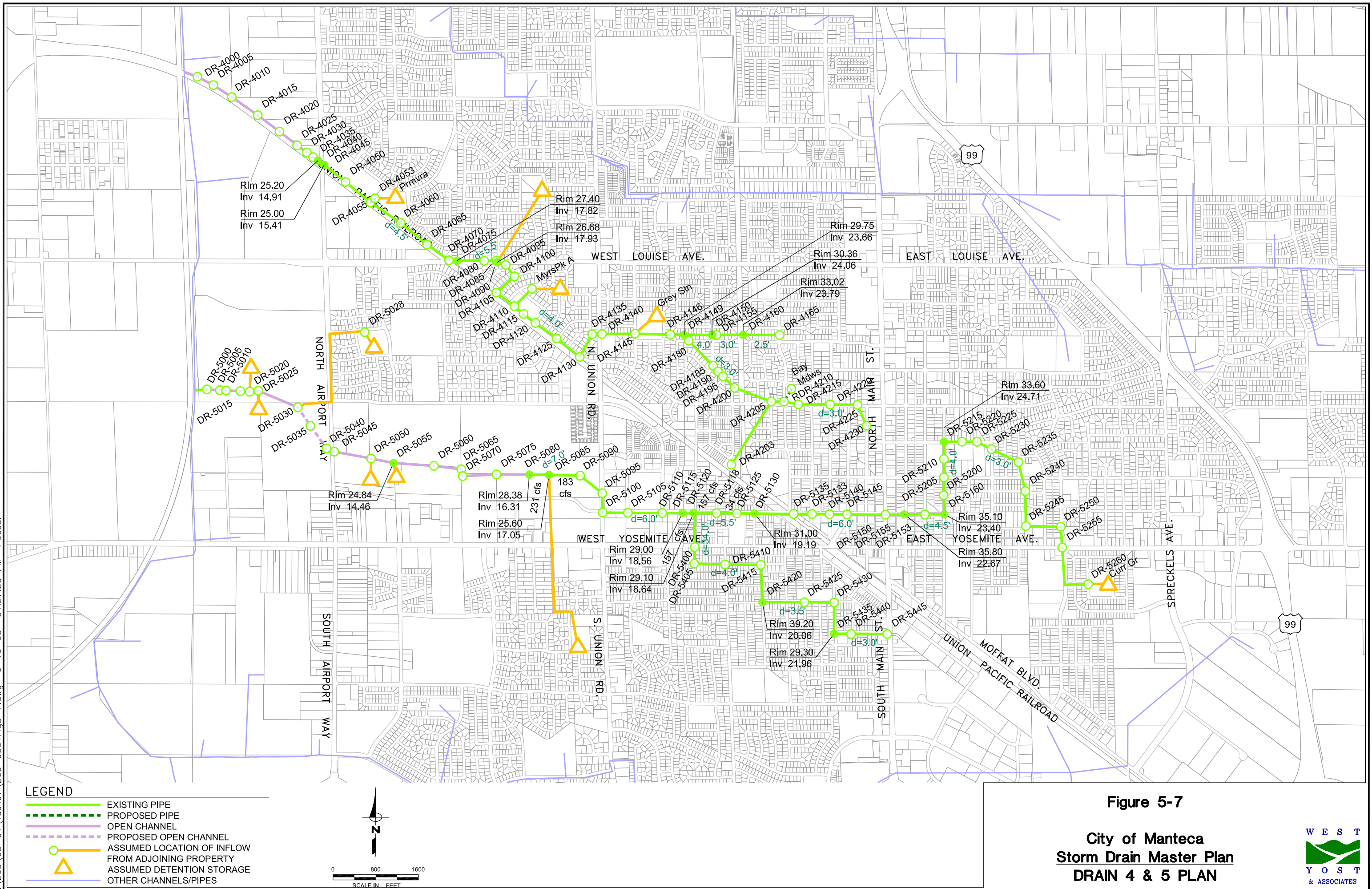
Results of Analysis

Figure 5-6 shows a schematic of Drain 4 with model nodes labeled. Figure 5-7 is a plan view of Drain 4. Table 5-6, Drain 4 Flows, show the peak flow rate computed at each model node. Table 5-7, Drain 4 Water Surface Elevations, shows model output data and flooding depths at each model node with a problem designation when flooding is computed above ground level.

City of Manteca
Storm Drain Master Plan
DRAIN 4 SCHEMATIC



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Conduit Name	Upstream Node	Downstream Node	Conduit Class	Conduit Type	Max Flow, ft ³ /s	Upstream Max Water Elevation, feet	Pipe Diameter/ Box Culvert Width, feet	Box Culvert Height, feet
DRP4160	DR-4165	DR-4160	Circular	PIPE	5.1	27.7	2.5	
DRP4155	DR-4160	DR-4155	Circular	PIPE	6.2	27.7	3.0	
DRP4150	DR-4155	DR-4150	Circular	PIPE	6.2	27.6	3.0	
DRP4149	DR-4150	DR-4149	Circular	PIPE	10.6	27.6	4.0	
DRP4225	DR-4230	DR-4225	Circular	PIPE	2.7	31.2	3.0	
DRP4220	DR-4225	DR-4220	Circular	PIPE	4.7	30.8	3.0	
DRP4215	DR-4220	DR-4215	Circular	PIPE	16.7	30.8	3.0	
DRP4210	DR-4215	RDR-4210	Circular	PIPE	20.2	30.3	3.0	
DRP4210A	BayMdws	RDR-4210	Circular	PIPE	2.5	30.1	1.0	
DRP4205	RDR-4210	DR-4205	Circular	PIPE	19.3	30.1	3.0	
DRP4205A	DR-4203	DR-4205	Circular	PIPE	2.7	30.8	1.3	
DRP4200	DR-4205	DR-4200	Circular	PIPE	27.6	30.0	3.0	
DRC4195	DR-4200	DR-4195	Natural	OPEN CHANNEL	20.4	29.3		
DRC4190	DR-4195	DR-4190	Natural	OPEN CHANNEL	21.3	29.3		
DRP4185	DR-4190	DR-4185	Circular	PIPE	25.6	29.3	3.0	
DRP4180	DR-4185	DR-4180	Circular	PIPE	25.6	28.9	3.0	
DRP4149A	DR-4180	DR-4149	Circular	PIPE	25.6	27.7	3.5	
DRP4146	DR-4149	DR-4146	Circular	PIPE	35.8	27.6	4.0	
DRP4145	DR-4146	DR-4145	Circular	PIPE	35.9	27.4	4.0	
DRP4140	DR-4145	DR-4140	Circular	PIPE	41.8	26.9	4.0	
DRP4135	DR-4140	DR-4135	Circular	PIPE	41.9	26.9	4.0	
DRP4130	DR-4135	DR-4130	Circular	PIPE	41.8	26.7	4.0	
DRP4125	DR-4130	DR-4125	Circular	PIPE	41.7	26.2	4.0	
DRP4120	DR-4125	DR-4120	Circular	PIPE	41.7	25.7	4.0	
DRP4115	DR-4120	DR-4115	Circular	PIPE	41.6	25.1	4.0	
DRP4110	DR-4115	DR-4110	Circular	PIPE	41.5	24.5	4.0	
DRP4105	DR-4110	DR-4105	Circular	PIPE	41.4	23.7	4.0	
DRP4100	DR-4105	DR-4100	Circular	PIPE	41.5	23.8	4.0	
DRP4095	DR-4100	DR-4095	Circular	PIPE	41.6	23.3	4.0	
DRP4090	DR-4095	DR-4090	Circular	PIPE	41.7	23.1	4.0	
DRP4090A	StFrancis	DR-4090	Circular	PIPE	2.6	22.1	1.5	
DRP4085	DR-4090	DR-4085	Circular	PIPE	53.4	23.0	4.0	
DRP4080	DR-4085	DR-4080	Circular	PIPE	53.3	23.0	5.5	
DRP4075	DR-4080	DR-4075	Circular	PIPE	53.1	22.9	5.5	
DRP4070	DR-4075	DR-4070	Circular	PIPE	53.0	22.8	4.5	
DRP4065	DR-4070	DR-4065	Circular	PIPE	53.0	22.7	4.5	
DRP4060	DR-4065	DR-4060	Circular	PIPE	53.0	22.3	4.5	
DRP4055	DR-4060	DR-4055	Circular	PIPE	53.0	21.6	4.5	
DRP4055A	DR-4053	DR-4055	Circular	PIPE	7.2	21.2	2.0	
DRP4050	DR-4055	DR-4050	Circular	PIPE	55.0	20.6	4.5	
DRP4045	DR-4050	DR-4045	Circular	PIPE	55.0	19.7	4.5	
DRP435	DR-4045	DR-4040	Circular	PIPE	54.9	19.6	5.0	
DRC4035	DR-4040	DR-4035	Natural	OPEN CHANNEL	54.9	19.6		
DRC4030	DR-4035	DR-4030	Natural	OPEN CHANNEL	54.9	19.6		
DRC4025	DR-4030	DR-4025	Natural	OPEN CHANNEL	54.4	19.6		
DRC4020	DR-4025	DR-4020	Natural	OPEN CHANNEL	53.4	19.6		
DRC4015	DR-4020	DR-4015	Natural	OPEN CHANNEL	52.8	19.6		
DRC4010	DR-4015	DR-4010	Natural	OPEN CHANNEL	52.0	19.6		
DRC4005	DR-4010	DR-4005	Natural	OPEN CHANNEL	51.2	19.6		
DRC4000	DR-4005	DR-4000	Natural	OPEN CHANNEL	50.2	19.6		

Table 5-7. Drain 4 - Water Surface Elevations

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev
DR-4160	33.02	23.79	27.7	32.7	-5.1	OK
DR-4155	30.2	23.01	27.6	29.9	-2.3	OK
DR-4150	30.36	24.06	27.6	30.1	-2.5	OK
DR-4230	35	30.542	31.2	34.7	-3.5	OK
DR-4225	35	29.118	30.8	34.7	-3.9	OK
DR-4220	35	28.638	30.8	34.7	-3.9	OK
DR-4215	35	26.902	30.3	34.7	-4.4	OK
BayMdws	33.5	29	30.1	33.2	-3.1	OK
RDR-4210	30	26.39	30.1	29.7	0.4	PROBLEM
WalnutPl	31.32	18.35	28.2	31.0	-2.8	OK
DR-4203	31.4	26.9	30.8	31.1	-0.3	OK
DR-4205	30	25.866	30.0	29.7	0.3	PROBLEM
DR-4200	31.82	24.39	29.3	30.5	-1.2	OK
DR-4195	32.54	25	29.3	29.5	-0.2	OK
DR-4190	31.16	24.28	29.3	30.9	-1.6	OK
DR-4185	30.2	24.01	28.9	29.9	-1.0	OK
DR-4180	30.05	22.57	27.7	29.8	-2.0	OK
DR-4149	29.75	23.66	27.6	29.5	-1.9	OK
DR-4146	29.45	23.454	27.4	29.2	-1.8	OK
GreyStn	29	14	25.7	28.7	-3.0	OK
DR-4145	29.14	23.05	26.9	28.8	-1.9	OK
DR-4140	29.57	22.57	26.9	29.3	-2.3	OK
DR-4135	28.2	22.6	26.7	27.9	-1.2	OK
DR-4130	29.3	22.54	26.2	29.0	-2.8	OK
DR-4125	29.57	22.17	25.7	29.3	-3.6	OK
DR-4120	27.86	22.05	25.1	27.6	-2.5	OK
DR-4115	29.42	21.69	24.5	29.1	-4.7	OK
MyrsPk A	26	9	26.6	25.7	0.9	PROBLEM
DR-4110	29.24	21.66	23.7	28.9	-5.2	OK
DR-4105	29.24	21.44	23.8	28.9	-5.1	OK
DR-4100	26.31	19.41	23.3	26.0	-2.7	OK
DR-4095	27.82	19.02	23.1	27.5	-4.4	OK
StFrancs	26	20	22.2	25.7	-3.6	OK
DR-4090	27.24	19.06	23.0	26.9	-4.0	OK
DR-4085	26.68	17.93	23.0	26.4	-3.4	OK
DR-4080	26.1	18.12	22.9	25.8	-2.9	OK
DR-4075	27.4	17.82	22.8	27.1	-4.3	OK
DR-4070	27	17.75	22.7	26.7	-4.0	OK
DR-4065	25.8	17.37	22.3	25.5	-3.2	OK
DR-4060	25	16.82	21.6	24.7	-3.1	OK
Prmvra	22.61	7.38	18.4	22.3	-3.9	OK
DR-4053	24	20.25	21.2	23.7	-2.5	OK
DR-4055	25	18.34	20.6	24.7	-4.1	OK
DR-4050	23.4	15.86	19.7	23.1	-3.4	OK
DR-4045	25	15.41	19.6	24.7	-5.1	OK
DR-4040	25.2	14.91	19.6	24.9	-5.3	OK
DR-4035	24.16	14.4	19.6	23.7	-4.0	OK
DR-4030	24.16	14.1	19.6	23.7	-4.0	OK
DR-4025	22.83	12.9	19.6	22.7	-3.1	OK
DR-4020	22.8	13.23	19.6	21.1	-1.4	OK
DR-4015	23.59	13.31	19.6	23.1	-3.4	OK
DR-4010	22.67	12.62	19.6	22.0	-2.3	OK
DR-4005	21.63	12.06	19.6	21.6	-2.0	OK
DR-4000	19.27	10.69	19.6	19.2	0.4	PROBLEM

The Mayor's Park area shows flooding up to two feet. This area is a good candidate for pump optimization analyses with two pump stations and the park basin. Unrestricted flows from upstream trigger the control system and shut down Mayors Park pumps causing minor flooding. The Drain 4 central City area does not have the detention basins that exist in much of the City.

Table 5-7 shows three locations where there is brief flooding up to two inches and four locations where the City's design criteria is not met by a couple of inches. These limited locations are likely not serious enough to warrant Master Plan solutions. The City can live with this minor short term nuisance. They do, however, demonstrate that Drain 4 is near capacity and does not have capacity for additional development or additional inflows.

DRAIN 5

Drain 5 begins as a 36-inch diameter pipe along South Street and increases to 42 inches as it turns into Park Avenue. The drain turns west on Oregon Street and increases to 48 inches turning north on South Veach Street. It then follows west along Nevada Street and north on Walnut Street to Center Street where it connects with the Drain 5 Interceptor.

Drain 5 Interceptor

The Drain 5 Interceptor begins at Curran Grove as a 36-inch pipe, proceeds north on Powers, north on Washington and west along North turning south in a 48 inch pipe to Center Street. The Interceptor flows west along Center Street to Sycamore as a 54-inch increasing to 60 inches to South Veach Street and 66 inches to Walnut. After the confluence with Drain 5 at Walnut and Center Parkway, the Interceptor continues west in a 72-inch diameter pipe connecting to a 60-inch pipe along the golf course. From the golf course the Interceptor flows west in an open drainage channel crossing under Airport Way in a 6-foot by 3-foot box culvert, that appears to be inadequate, then passes through two parallel 48-inch pipes for 1,000 feet and continues in an open channel to its outfall to the FCOC in a 84-inch corrugated metal pipe. The low area upstream of the FCOC functions as storage for Drain 5.

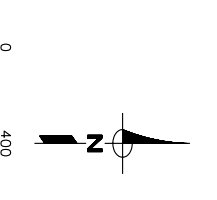
Results of Analysis

Figure 5-8 shows a schematic of Drain 5 with model nodes labeled. Table 5-8, Drain 5 Flows, shows the peak flow rate and the maximum water surface elevation computed at each model node. Table 5-9, Drain 5 Water Surface Elevations, shows model output data and flooding depths at each model node with a problem designation when flooding is computed above ground level. Figure 5-7 shows a plan view of Drain 5.

The model output also shows some areas of minor short term ponding that may not warrant the construction of extensive improvement projects. The model output, Table 5-9, also shows long stretches of the lower half of Drain 5 where City standards are violated. These are areas where the hydraulic grade line is not 0.5 foot below rim elevation.

Figure 5-8

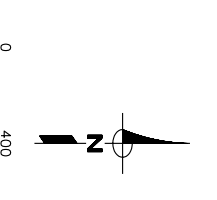
City of Manteca
Storm Drain Master Plan
DRAIN 5 SCHEMATIC



W E S T
Y O S T
& ASSOCIATES

Figure 5-8

City of Manteca
Storm Drain Master Plan
DRAIN 5 SCHEMATIC



W E S T
Y O S T
& ASSOCIATES

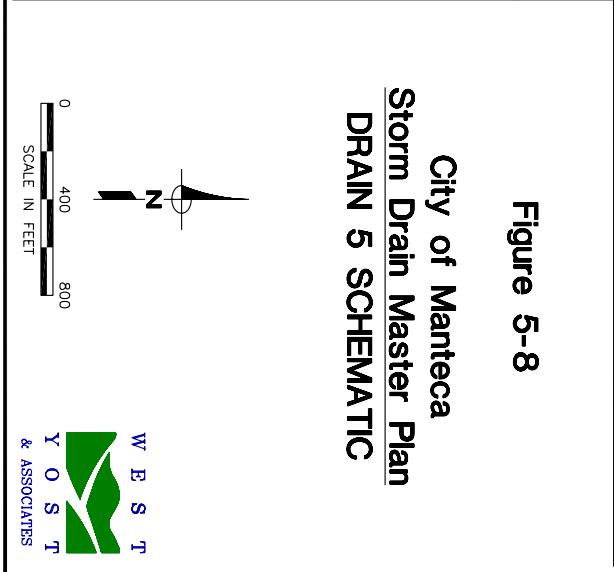


Table 5-8. Drain 5 - Flows

Upstream Node	Downstream Node	Conduit Class	Conduit Type	Max Flow, ft ³ /s	Upstream Max Water Elevation, feet	Pipe Diameter/ Box Culvert Width, feet	Box Culvert Height, feet
DR-5445	DR-5440	Circular	PIPE	34.2	31.4	3.0	
DR-5440	DR-5435	Circular	PIPE	22.4	28.8	3.0	
DR-5435	DR-5430	Circular	PIPE	24.4	28.8	3.5	
DR-5430	DR-5425	Circular	PIPE	24.7	28.8	3.5	
DR-5425	DR-5420	Circular	PIPE	25.9	28.8	3.5	
DR-5420	DR-5415	Circular	PIPE	26.8	28.9	4.0	
DR-5415	DR-5410	Circular	PIPE	28.0	28.9	4.0	
DR-5410	DR-5405	Circular	PIPE	28.0	28.8	4.0	
DR-5405	DR-5400	Circular	PIPE	33.8	29.1	4.0	
DR-5400	DR-5120	Circular	PIPE	33.8	28.9	4.0	
DR-5260	DR-5255	Circular	PIPE	3.3	36.9	3.0	
DR-5255	DR-5250	Circular	PIPE	11.9	36.6	3.0	
DR-5250	DR-5245	Circular	PIPE	17.5	36.5	3.0	
DR-5245	DR-5240	Circular	PIPE	24.9	36.1	3.0	
DR-5240	DR-5235	Circular	PIPE	23.9	35.0	3.0	
DR-5235	DR-5230	Circular	PIPE	24.6	34.5	3.0	
DR-5230	DR-5225	Circular	PIPE	37.2	33.5	3.0	
DR-5225	DR-5220	Circular	PIPE	38.7	33.5	3.0	
DR-5220	DR-5215	Circular	PIPE	38.8	33.5	3.0	
DR-5215	DR-5210	Circular	PIPE	54.3	33.5	4.0	
DR-5210	DR-5205	Circular	PIPE	53.5	33.5	4.0	
DR-5205	DR-5200	Circular	PIPE	56.3	33.4	4.0	
DR-5200	DR-5160	Circular	PIPE	56.3	33.4	4.0	
DR-5160	DR-5153	Circular	PIPE	61.6	33.8	4.5	
DR-5153	DR-5155	Circular	PIPE	66.8	33.8	4.5	
DR-5155	DR-5150	Circular	PIPE	83.8	33.6	5.0	
DR-5150	DR-5145	Circular	PIPE	77.6	33.5	5.0	
DR-5145	DR-5140	Circular	PIPE	78.9	33.6	5.0	
DR-5140	DR-5133	Circular	PIPE	89.6	32.6	5.0	
DR-5133	DR-5135	Circular	PIPE	93.8	32.3	5.0	
DR-5135	DR-5130	Circular	PIPE	95.0	31.5	5.0	
DR-5130	DR-5125	Circular	PIPE	121.5	30.3	5.5	
DR-5125	DR-5118	Circular	PIPE	121.1	29.4	5.5	
DR-5118	DR-5120	Circular	PIPE	130.9	28.9	5.5	
DR-5120	DR-5115	Circular	PIPE	157.8	28.3	5.5	
DR-5115	DR-5110	Circular	PIPE	156.8	27.9	6.0	
DR-5110	DR-5105	Circular	PIPE	145.1	27.1	6.0	
DR-5105	DR-5100	Circular	PIPE	167.6	26.4	6.0	
DR-5100	DR-5095	Circular	PIPE	168.2	25.3	6.0	
DR-5095	DR-5090	Circular	PIPE	177.6	24.7	6.0	
DR-5090	DR-5085	Circular	PIPE	175.8	23.5	6.0	
UnionWst	DR-5085	Circular	PIPE	8.2	24.4	3.0	
DR-5085	DR-5080	Circular	PIPE	221.1	22.2	7.0	
DR-5080	DR-5075	Natural	OPEN CHANNEL	215.9	20.8		
DR-5075	DR-5070	Natural	OPEN CHANNEL	212.4	20.7		
DR-5070	DR-5065	Natural	OPEN CHANNEL	211.0	20.7		
DR-5065	DR-5060	Natural	OPEN CHANNEL	209.9	20.7		
DR-5060	DR-5055	Natural	OPEN CHANNEL	208.8	20.7		
St Domnc	DR-5055	Circular	PIPE	0.6	20.7	1.0	
DR-5055	DR-5050	Natural	OPEN CHANNEL	207.9	20.7		
DR-5050	DR-5045	Natural	OPEN CHANNEL	207.2	20.7		
DR-5045	DR-5040	Rectangle	BOX CULVERT	103.5	20.7	6.0	3.0
DR-5045	DR-5040	Rectangle	BOX CULVERT	103.5	20.7	6.0	3.0
DR-5040	DR-5035	Trapezoid	OPEN CHANNEL	206.3	20.7		
DR-5035	DR-5030	Trapezoid	OPEN CHANNEL	204.7	20.7		
DR-5028	DR-5030	Circular	PIPE	0.0	20.3	2.5	
DR-5030	DR-5025	Natural	OPEN CHANNEL	77.0	20.0		
DR-5025	DR-5020	Natural	OPEN CHANNEL	75.9	20.0		
DR-5020	DR-5015	Natural	OPEN CHANNEL	75.3	20.0		
DR-5015	DR-5010	Natural	OPEN CHANNEL	74.6	20.0		
DR-5010	DR-5005	Natural	OPEN CHANNEL	73.2	20.0		
DR-5005	DR-5000	Natural	OPEN CHANNEL	74.2	20.0		
DR-5000	FC-70	Circular	PIPE	72.0	20.0	7.0	

Table 5-9. Drain 5 - Water Surface Elevations

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev
DR-5440	28.5	22.16	28.8	28.2	0.6	PROBLEM
DR-5435	29.3	21.96	28.8	29.0	-0.2	PROBLEM
DR-5430	31.5	21.1	28.8	31.2	-2.4	OK
DR-5425	29.5	20.67	28.8	29.2	-0.4	OK
DR-5420	30.2	20.06	28.9	29.9	-1.0	OK
DR-5415	30.7	19.76	28.9	30.4	-1.5	OK
DR-5410	29.24	19.46	28.8	28.9	-0.2	PROBLEM
DR-5405	30.1	19.21	29.1	29.8	-0.7	OK
DR-5400	30	18.86	28.9	29.7	-0.8	OK
Curr Gr	36.5	14	31.2	36.2	-5.0	OK
DR-5260	40	36.68	36.9	39.7	-2.8	OK
DR-5255	40	33.486	36.6	39.7	-3.1	OK
DR-5250	40	32.349	36.5	39.7	-3.2	OK
DR-5245	40	30.564	36.1	39.7	-3.6	OK
DR-5240	35	28.666	35.0	34.7	0.3	PROBLEM
DR-5235	34.8	27	34.5	34.5	0.0	PROBLEM
DR-5230	33.2	26.7	33.5	32.9	0.6	PROBLEM
DR-5225	34.3	27.1	33.5	34.0	-0.5	OK
DR-5220	34.4	25.66	33.5	34.1	-0.6	OK
DR-5215	33.6	24.71	33.5	33.3	0.2	PROBLEM
DR-5210	33.6	24.44	33.5	33.3	0.2	PROBLEM
DR-5205	34.6	24.06	33.4	34.3	-0.9	OK
DR-5200	33.8	23.82	33.4	33.5	-0.1	PROBLEM
DR-5160	35.1	23.4	33.8	34.8	-1.0	OK
DR-5153	35.45	23.04	33.8	35.2	-1.3	OK
DR-5155	35.8	22.67	33.6	35.5	-1.9	OK
DR-5150	34.9	22.32	33.5	34.6	-1.1	OK
DR-5145	34.1	21.56	33.6	33.8	-0.2	OK
DR-5140	33.1	21.24	32.6	32.8	-0.2	PROBLEM
DR-5133	33.1	20.94	32.3	32.8	-0.5	OK
DR-5135	33.1	20.63	31.5	32.8	-1.3	OK
DR-5130	31	19.19	30.3	30.7	-0.4	OK
DR-5125	30	18.96	29.4	29.7	-0.3	OK
DR-5118	29.55	18.8	28.9	29.3	-0.4	OK
DR-5120	29.1	18.64	28.3	28.8	-0.5	OK
DR-5115	29	18.56	27.9	28.7	-0.8	OK
DR-5110	27.1	18.32	27.1	26.8	0.3	PROBLEM
DR-5105	26.9	18	26.4	26.6	-0.2	PROBLEM
DR-5100	26.3	15	25.3	26.0	-0.7	OK
DR-5095	26.1	17.63	24.7	25.8	-1.1	OK
DR-5090	26.5	17.32	23.5	26.2	-2.7	OK
UnionWst	29	23.09	24.4	28.7	-4.3	OK
DR-5085	25.6	17.05	22.2	25.3	-3.1	OK
DR-5080	28.38	16.31	20.8	24.6	-3.8	OK
DR-5075	26.88	16.04	20.7	24.6	-3.9	OK
DR-5070	25.76	15.98	20.7	24.4	-3.7	OK
DR-5065	25.27	16.01	20.7	22.6	-1.9	OK
DR-5060	25.03	14.79	20.7	24.1	-3.4	OK
St Domnc	23	14.74	20.7	22.7	-2.0	OK
DR-5055	24.84	14.46	20.7	24.5	-3.8	OK

Table 5-9. Drain 5 - Water Surface Elevations, cont'd...

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev
DR-5050	24.84	14.35	20.7	24.2	-3.5	OK
DR-5045	23.96	14.3	20.7	23.7	-3.0	OK
DR-5040	25	14.24	20.7	21.7	-1.0	OK
DR-5035	25	13.66	20.7	21.7	-1.0	OK
Vil Tcno	23.7	7	20.6	23.4	-2.8	OK
DR-5028	24.3	20.3	20.3	24.0	-3.7	OK
DR-5030	25	13.1	20.0	21.7	-1.7	OK
DR-5025	21.87	13.06	20.0	21.9	-1.8	OK
DR-5020	20.85	12.87	20.0	20.3	-0.2	OK
DR-5015	20.85	12.789	20.0	20.3	-0.2	OK
DR-5010	21.84	12.78	20.0	21.5	-1.5	OK
DR-5005	21.84	8.88	20.0	20.3	-0.2	OK
DR-5000	20.95	8.78	20.0	21.0	-0.9	OK

The Airport Way culvert is undersized. The recommendation is to matching the existing 6-foot by 3-foot box culvert with a second box. An analysis with a second Airport Way culvert and replacement of the two 48-inch parallel CMP drains with an open drainage channel shows that flooding problems are alleviated. This solution is similar to previous Master Plan updates and is strongly recommended.

Master Plan Project

- Airport Way Construct 6-foot by 3-foot box culvert to match existing culvert
- Downstream of Convert 1000 feet of two 48-inch pipes to open drainage channel
Airport Way

DRAIN 7

Drain 7 begins on the east side of Spreckels Avenue where runoff flows into the first of two Sprekels detention basins. Runoff is pumped out of the first basin into the second basin. A 24-inch diameter force main will soon convey flow pumped from the second basin west to Main Street. Two new inflows to Drain 7 will serve an 84 acre mixed use land area to the north and a 221 acre industrial area along Industrial Park Drive and pump into the force main. Runoff now going to Pump Stations 8 and 9 will be diverted to one of the new detention basins north of Industrial Park Drive. Outflow from the basin will be pumped into the Drain 7 force main. All four Spreckels / Industrial basins and pumps will have monitored discharges to maintain flow limits downstream.

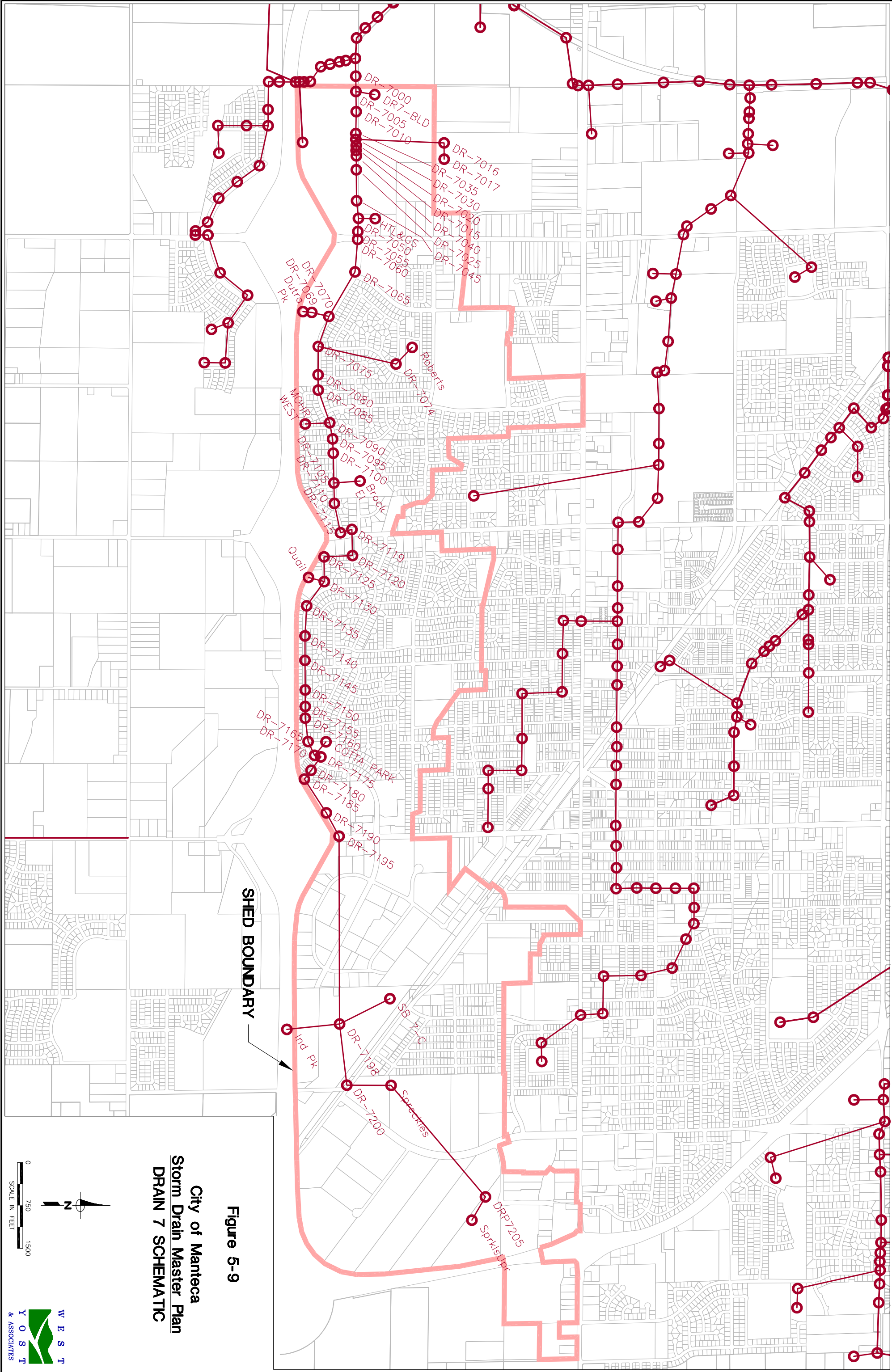
The force main ends at Main Street and Drain 7 continues west in a 36-inch pipe. The drain follows a little to the north of Highway 120 with an increase to 42 inches opposite Stonum Lane. The 42-inch pipe continues to Airport Way where a 3-foot by 10-foot box culvert is under constructed to carry Drain 7 under Daniels Street by the Big League Dreams complex. The box culvert ends at the confluence with Drain 8 at the beginning of the French Camp Outlet Canal.

Drain 7 is well controlled with 10 detention basins along its length. More detailed system operation studies may help to make the system of basins and pumps work more effectively and reduce some of the flooding along Drain 7.

Drain 7 flows through the center of the City's Big League Dreams (BLD) project and the channel is being moved to a 12-foot by 10-foot box culvert that runs under Daniels Street from Airport Way to the west boundary of BLD, 1,300 feet to the west.

Results of Analysis

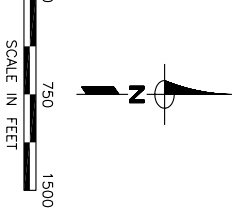
Figure 5-9 shows a schematic of Drain 7 with model nodes labeled. Table 5-10, Drain 7 Flows, shows the peak flow rate computed at each model node. Table 5-11, Drain 7 Water Surface Elevations, shows model output data and flooding depths at each model node with a problem designation when flooding is computed above ground level. Figure 5-10 is a plan view of Drain 7.



SHED BOUNDARY

Figure 5-9

City of Manteca
Storm Drain Master Plan
DRAIN 7 SCHEMATIC



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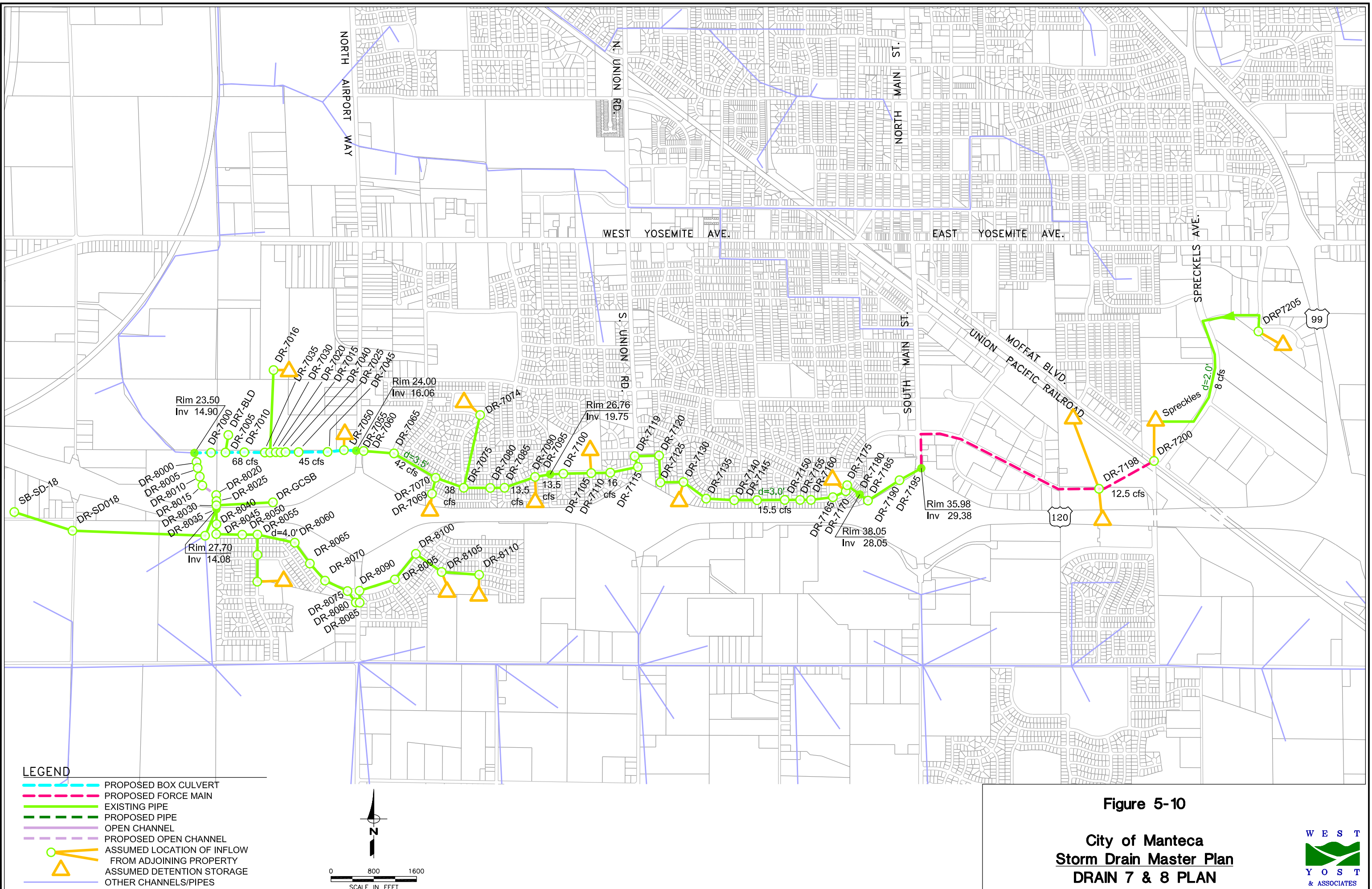


Table 5-10. Drain 7 - Flows

Upstream Node	Downstream Node	Conduit Class	Conduit Type	Max Flow, ft ³ /s	Upstream Max Water Elevation, feet	Pipe Diameter/ Box Culvert Width, feet	Box Culvert Height, feet
DRP7205	Spreckles	Circular	PIPE	7.8	41.3	2.0	
DR-7200	DR-7198	Circular	FORCEMAIN	12.5	34.5	2.0	
DR-7198	DR-7195	Circular	FORCEMAIN	12.1	34.0	2.0	
DR-7195	DR-7190	Circular	PIPE	12.0	31.2	3.5	
DR-7190	DR-7185	Circular	PIPE	12.0	30.9	3.5	
DR-7185	DR-7180	Circular	PIPE	12.0	30.2	3.5	
DR-7180	DR-7175	Circular	PIPE	12.1	30.1	3.0	
DR-7175	DR-7170	Circular	PIPE	12.3	29.7	3.0	
DR-7170	DR-7165	Circular	PIPE	12.5	29.5	3.0	
DR-7165	DR-7160	Circular	PIPE	13.0	29.4	3.0	
DR-7160	DR-7155	Circular	PIPE	13.3	29.2	3.0	
DR-7155	DR-7150	Circular	PIPE	13.7	29.1	3.0	
DR-7150	DR-7145	Circular	PIPE	14.0	29.0	3.0	
DR-7145	DR-7140	Circular	PIPE	14.3	28.8	3.0	
DR-7140	DR-7135	Circular	PIPE	14.7	28.6	3.0	
DR-7135	DR-7130	Circular	PIPE	15.7	28.4	3.0	
DR-7130	DR-7125	Circular	PIPE	16.1	28.2	3.0	
DR-7125	DR-7120	Circular	PIPE	16.1	27.8	3.0	
DR-7120	DR-7119	Circular	PIPE	16.1	27.4	3.0	
DR-7119	DR-7115	Circular	PIPE	16.1	27.1	3.0	
DR-7115	DR-7110	Circular	PIPE	16.1	26.9	3.0	
DR-7110	DR-7105	Circular	PIPE	16.1	26.5	3.0	
Brock El	DR-7105	Circular	PIPE	5.0	24.2	1.5	
DR-7105	DR-7100	Circular	PIPE	11.2	26.3	3.0	
DR-7100	DR-7095	Circular	PIPE	11.2	26.1	3.0	
DR-7095	DR-7090	Circular	PIPE	11.2	26.0	3.5	
MOHR WEST	DR-7090	Circular	PIPE	2.1	26.5	1.0	
DR-7090	DR-7085	Circular	PIPE	11.2	25.9	3.5	
DR-7085	DR-7080	Circular	PIPE	36.2	25.8	3.5	
DR-7080	DR-7075	Circular	PIPE	36.2	25.6	3.5	
DR-7074	DR-7075	Circular	PIPE	2.8	25.8	1.5	
DR-7075	DR-7070	Circular	PIPE	38.0	24.5	3.5	
DR-7069	DR-7070	Circular	PIPE	2.8	23.9	1.5	
DR-7070	DR-7065	Circular	PIPE	40.0	23.8	3.5	
DR-7065	DR-7060	Circular	PIPE	40.0	22.7	3.5	
DR-7060	DR-7055	Circular	PIPE	40.1	21.7	3.5	
DR-7055	DR-7050	Rectangle	BOX CULVERT	40.1	21.5	8.0	3.0
DR-7050	DR-7045	Rectangle	BOX CULVERT	40.1	21.4	8.0	3.0
DR-7045	DR-7040	Rectangle	BOX CULVERT	40.0	21.4	8.0	3.0
DR-7040	DR-7020	#N/A	BOX CULVERT	40.1	21.3	8.0	3.0
DR-7020	DR-7010	#N/A	BOX CULVERT	65.1	21.2	12.0	3.0
DR-7010	DR-7005	Rectangle	BOX CULVERT	65.1	21.1	12.0	3.0
DR-7005	DR-7000	Rectangle	BOX CULVERT	65.6	21.0	12.0	3.0
DR-7000	FC-25	Natural	OPEN CHANNEL	69.2	21.0		

Table 5-11. Drain 7 - Water Surface Elevations

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev	Notes
SprklsUpr	40.6	21	37.2	40.3	-3.1	OK	
DRP7205	41	35.91	41.3	40.7	0.6	PROBLEM	
Spreckles	38.18	13.79	36.7	37.9	-1.2	OK	
DR-7200	40	29.38	34.5	39.7	-5.2	OK	
SB 7-C	40	29.6	35.1	39.7	-4.6	OK	
DR-7198	40	29.26	34.0	39.7	-5.7	OK	
DR-7195	35.98	29.38	31.2	35.7	-4.5	OK	
DR-7190	33.61	29.23	30.9	33.3	-2.4	OK	Low Spot
DR-7185	39.25	28.69	30.2	39.0	-8.7	OK	
DR-7180	38.05	28.05	30.1	37.8	-7.7	OK	
DR-7175	33.51	28.19	29.7	33.2	-3.6	OK	Low Spot
COTTA PARK	33	12.5	28.4	32.7	-4.3	OK	
DR-7170	33.26	27.36	29.5	33.0	-3.4	OK	Low Spot
DR-7165	34.13	27.23	29.4	33.8	-4.4	OK	
DR-7160	32.66	26.45	29.2	32.4	-3.2	OK	Low Spot
DR-7155	33.45	26.45	29.1	33.2	-4.0	OK	
DR-7150	32.29	26.04	29.0	32.0	-3.0	OK	Low Spot
DR-7145	32.87	25.77	28.8	32.6	-3.8	OK	
DR-7140	31.64	25.44	28.6	31.3	-2.7	OK	
DR-7135	30.47	24.87	28.4	30.2	-1.8	OK	
Quail	29	12.16	27.5	28.7	-1.2	OK	
DR-7130	30.47	24.21	28.2	30.2	-2.0	OK	
DR-7125	29.73	23.63	27.8	29.4	-1.6	OK	
DR-7120	29.34	23.04	27.4	29.0	-1.6	OK	
DR-7119	35.35	22.36	27.1	35.1	-7.9	OK	
DR-7115	33	22.2	26.9	32.7	-5.8	OK	
DR-7110	29.8	21.67	26.5	29.5	-3.0	OK	
Brock El	27.57	24.17	24.2	27.3	-3.1	OK	
DR-7105	27.8	21.31	26.3	27.5	-1.2	OK	
DR-7100	26.55	21.23	26.1	26.3	-0.2	PROBLEM	Low Spot
DR-7095	27.1	21	26.0	26.8	-0.8	OK	
MOHR WEST	26.76	21.653	26.5	26.5	0.1	PROBLEM	
DR-7090	26.76	19.75	25.9	26.5	-0.5	OK	
DR-7085	26	19.12	25.8	25.7	0.1	PROBLEM	
DR-7080	26	18.99	25.6	25.7	-0.1	PROBLEM	
Roberts	24.5	10.37	22.0	24.2	-2.2	OK	
DR-7074	25.2	20.99	25.8	24.9	0.9	PROBLEM	
DR-7075	24.2	18.43	24.5	23.9	0.6	PROBLEM	
DutraPk	24.5	8.63	19.1	24.2	-5.1	OK	
DR-7069	24.5	17.5	23.9	24.2	-0.3	OK	
DR-7070	23	17.84	23.8	22.7	1.1	PROBLEM	Low Spot
DR-7065	25.2	17.09	22.7	24.9	-2.2	OK	
DR-7060	23.6	16.46	21.7	23.3	-1.6	OK	
DR-7055	23.48	16.06	21.5	24.0	-2.5	OK	
DR-7050	24	15.97	21.4	24.0	-2.6	OK	
DR-7045	24	15.85	21.4	24.0	-2.6	OK	
DR-7040	24	15.61	21.3	24.0	-2.7	OK	
DR-7020	24	15.46	21.2	24.0	-2.8	OK	
DR-7010	24	15.23	21.1	24.0	-2.9	OK	
DR-7005	24	15.04	21.0	24.0	-3.0	OK	
DR-7000	24	14.99	21.0	24.0	-3.0	OK	
DR7-BLD	23.36	15.25	18.7	23.1	-4.3	OK	
FC-25	23.5	14.9	21.0	21.5	-0.6	OK	

As the industrial zoned land along Industrial Park Drive is developed, the City's plan is to construct a force main from the west Spreckels basin to South Main Street. There are two basins with a total pumping capacity to the drain of 16 cfs. Two more basins will serve the land between Spreckels and Main Street. All four of these basins will provide extended storage and runoff will be held in the basin for a longer time to avoid flooding at and downstream of South Main Street.

Master Plan Projects

A force main required from the industrial area to Main Street and the improvements associated with the Daniels Street Improvements west of Airport Way are key projects along Drain 7.

- Industrial Park Drive Force main, 24-inch, from Spreckels pump to Main Street
- Daniels Street Box Culvert

DRAIN 8

Most south area lands within the Drain 8 drainage area will drain to the South Drain after that facility is constructed and land is developed. Under the Master Plan, about 236 acres of residential land immediately south of Highway 120 will continue to drain to Drain 8 and flow north to its confluence with Drain 7 at the beginning of the FCOC. A 4.5 acre wetland along the east side of Drain 8 will be protected as important wetland habitat.

Drain 8 will also receive some South Drain flow pumped into the channel on the north side of Highway 120. The combined flow in Drain 8 from the South Drain pump station and from gravity flow in Drain 8 from south of Highway 120 will be 125 cfs. The Drain 8 channel will have to be widened by 10 feet along its west side to avoid the wetlands east of the channel.

A small road crossing north of the highway would have to be reconstructed and a 4-foot by 8-foot box culvert constructed.

Results of Analysis

Figure 5-11 shows a schematic of Drain 8 with model nodes labeled. Table 5-12, Drain 8 Flows, shows the peak flow rate and the maximum water surface elevation computed at each model node. Table 5-13, Drain 8 Water Surface Elevations, shows model output data and flooding depths at each model node with a problem designation when flooding is computed above ground level. A plan of Drain 8 is shown in Figure 5-10.

Master Plan Projects

These recommended improvements for Drain 8 include the following facilities:

	Location	Recommended Improvement
•	Road crossing, 250 ft north of Hwy 120 with a 4-foot by 8-foot box culvert	Replace existing crossing and culvert
•	Drain 8, Hwy 120 to FCOC	Widen channel 10 feet along west side

Figure 5-11

City of Manteca
Storm Drain Master Plan
DRAIN 8 SCHEMATIC

The schematic map shows the layout of Drain 8, including its main channel and various tributaries. The map is oriented with North at the top. A scale bar indicates distances up to 800 feet. The legend identifies the map as being prepared by WEST YOST & ASSOCIATES.

Figure 5-11

City of Manteca
Storm Drain Master Plan
DRAIN 8 SCHEMATIC

The schematic map shows the layout of Drain 8, including its main channel and various tributaries. The map is oriented with North at the top. A scale bar indicates distances up to 800 feet. The legend identifies the map as being prepared by WEST YOST & ASSOCIATES.

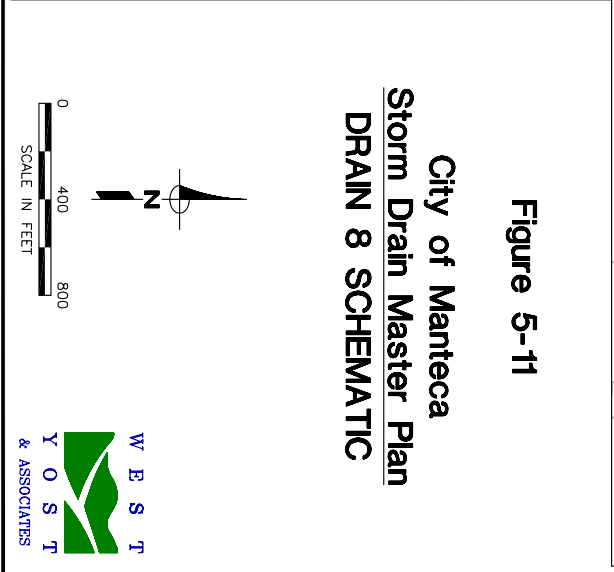


Table 5-12. Drain 8 - Flows

Upstream Node	Downstream Node	Conduit Class	Conduit Type	Max Flow, ft ³ /s	Upstream Max Water Elevation, feet	Pipe Diameter/ Box Culvert Width, feet	Box Culvert Height, feet
DR-8115	DR-8110	Circular	PIPE	1.3	21.3	4.0	
DR-8110	DR-8105	Circular	PIPE	2.9	21.3	4.0	
DR-8105	DR-8100	Circular	PIPE	5.5	21.3	4.0	
DR-8100	DR-8095	Circular	PIPE	5.8	21.3	4.0	
DR-8095	DR-8090	Circular	PIPE	6.2	21.2	4.0	
DR-8090	DR-8085	Circular	PIPE	6.3	21.2	4.0	
DR-8085	DR-8080	Rectangle	BOX CULVERT	6.4	21.2	6.0	2.5
DR-8080	DR-8075	Circular	PIPE	6.5	21.2	4.0	
DR-8075	DR-8070	Circular	PIPE	6.6	21.2	4.0	
DR-8070	DR-8065	Circular	PIPE	6.7	21.2	4.0	
DR-8065	DR-8060	Circular	PIPE	7.0	21.2	4.0	
DR-8060	DR-8055	Circular	PIPE	7.3	21.2	4.0	
DR-8005A	DR-8000A	Circular	PIPE	14.4	21.2	4.0	
DR-8000A	DR-8055	Circular	PIPE	15.5	21.2	4.0	
DR-8055	DR-8050	Circular	PIPE	16.9	21.2	5.5	
DR-8050	DR-8045	Circular	PIPE	10.7	21.2	5.5	
DR-8045	DR-8040	Circular	PIPE	12.0	21.2	4.0	
DR-8040	DR-8035	Circular	PIPE	47.2	21.2	4.0	
DR-8035	DR-8030	Natural	OPEN CHANNEL	124.2	21.1		
DR-8030	DR-8025	Natural	OPEN CHANNEL	130.7	21.1		
DR-8025	DR-8020	Natural	OPEN CHANNEL	133.6	21.1		
DR-8015	DR-8020	Rectangle	BOX CULVERT	134.9	21.1	8.0	
DR-8015	DR-8010	Natural	OPEN CHANNEL	134.5	21.0		
DR-8010	DR-8005	Natural	OPEN CHANNEL	140.0	21.0		
DR-8005	DR-8000	Natural	OPEN CHANNEL	135.8	21.0		
DR-8000	FC-25	Natural	OPEN CHANNEL	134.1	21.0		

Table 5-13. Drain 8 - Water Surface Elevations

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev
DR-8115	25.2	17.31	21.3	24.9	-3.6	OK
DR-8110	23.5	17.15	21.3	23.2	-1.9	OK
Dutra Pk	20.88	4	14.2	20.6	-6.4	OK
DR-8105	24.5	16.87	21.3	24.2	-2.9	OK
DR-8100	23.5	16.61	21.3	23.2	-1.9	OK
DR-8095	23	16.36	21.2	22.7	-1.5	OK
DR-8090	23	16.08	21.2	21.6	-0.4	OK
DR-8085	23	16.03	21.2	21.5	-0.2	OK
DR-8080	23	15.14	21.2	21.1	0.1	PROBLEM
DR-8075	23	15.07	21.2	21.1	0.1	PROBLEM
DR-8070	23	14.92	21.2	22.5	-1.3	OK
DR-8065	22.8	14.74	21.2	20.9	0.3	PROBLEM
DR-8060	22.8	14.56	21.2	20.7	0.5	PROBLEM
BellaVsta	18	1.96	12.7	17.7	-5.0	OK
DR-8005A	23.5	13.62	21.2	20.3	0.9	PROBLEM
DR-8000A	23.5	13.46	21.2	20.7	0.5	PROBLEM
DR-8055	22.8	13.29	21.2	21.7	-0.5	OK
DR-8050	22	13.18	21.2	21.7	-0.5	OK
DR-8045	23.2	13.06	21.2	22.9	-1.7	OK
DR-8040	27.7	13.63	21.2	27.4	-6.2	OK
DR-8035	27.7	14.08	21.1	27.4	-6.3	OK
DR-GCSB	22	12	16.0	21.7	-5.7	OK
DR-8030	22.71	14.1	21.1	21.1	0.0	PROBLEM
DR-8025	22.71	14.09	21.1	21.1	0.0	PROBLEM
DR-8020	22.76	14.72	21.1	22.5	-1.4	OK
DR-8015	22.71	14.87	21.0	21.1	-0.1	PROBLEM
DR-8010	22.2	14.67	21.0	21.6	-0.6	OK
DR-8005	22.2	15.32	21.0	19.7	1.3	PROBLEM
DR-8000	21.76	15.08	21.0	19.7	1.3	PROBLEM

SOUTH DRAIN

The Manteca General Plan designates a large area in south Manteca for development. The South Manteca plan area extends from the western city limits east past Highway 99 and is bounded on the north by Highway 120 and on the south by the city limits (see Figure 5-12). The area also includes land to the east to Austin Road between Highway 99 and one half mile north of Yosemite Avenue.

The South Drain was planned to serve these 7,680 acres of developing land. The land has historically been in agricultural land uses but is rapidly developing into primarily residential housing, commercial and industrial uses. The South Drain begins north of Yosemite Avenue and runs south to Highway 99. The South Drain turns west crossing under the highway and the Union Pacific Railroad tracks and continues along Woodward Road to Airport Way. The South Drain splits at Airport Way with early stages of development conveyed west along Woodward Avenue and north to Highway 120 and pumped into Drain 8. Drainage from later stages of development will be conveyed south along Airport Way and pumped into Drain 10. Drainage flows will be carried in Drain 10 and connect to Drain 11 and outfall to the San Joaquin River.

The Manteca south area is designated Drainage Financing Zone 36. The South Drain is planned to be the primary drainage facility for Zone 36. The City adopted a policy for development of the South Area Master Plan that at all stages of development, 10-year storm drainage protection will be provided by diverting all storm runoff to detention basins before being discharged to a terminal drainage facility. Detention basins are to provide attenuation of peak runoff and water quality treatment consistent with the City's NPDES permit. Basins and pump stations for the South Drain area will be sized in accordance with City standards as presented in Chapter 3, Policies and Design Criteria.

The South Drain was sized using the City XP-SWMM model prepared for the Master Plan. Figure 5-12 shows a schematic of the XP-SWMM model for South Drain. Lands draining to the South Drain and their land uses as designated by the General Plan and shown in Figure 5-13 and tabulated in Tables 5-14 and 5-15. The phasing of South Drain construction continues to follow the 1993 plan.

The map of Figure 5-14 shows the five major components of the South Drain system:

- The primary South Drain begins north of Yosemite Avenue, follows Austin Avenue south to Woodward Avenue and turns west along Woodward Avenue to Airport Way. The north-south leg begins with a 30 inch diameter pipe and increases to 42 inches as it crosses under Highway 99, Moffat Boulevard and the Union Pacific Railroad tracks.
- A southern tributary, 60 inches in diameter, beginning in the proposed industrial area, traversing west along a future road designated in the General Plan and north along South Main Street connecting with the South Drain.
- The north diversion of 66 inches in diameter that would bring a portion of South Drain flow from Airport Way west along Woodward Avenue and north along the alignment of the proposed road designated in the General Plan to Highway 120. A pump station would pump flow through a force main to Drain 8 and to the FCOC.
- A 36 inch increasing to 48 inch pipe tributary that would convey flows from the west near Oakwood Lake east to the north diversion.

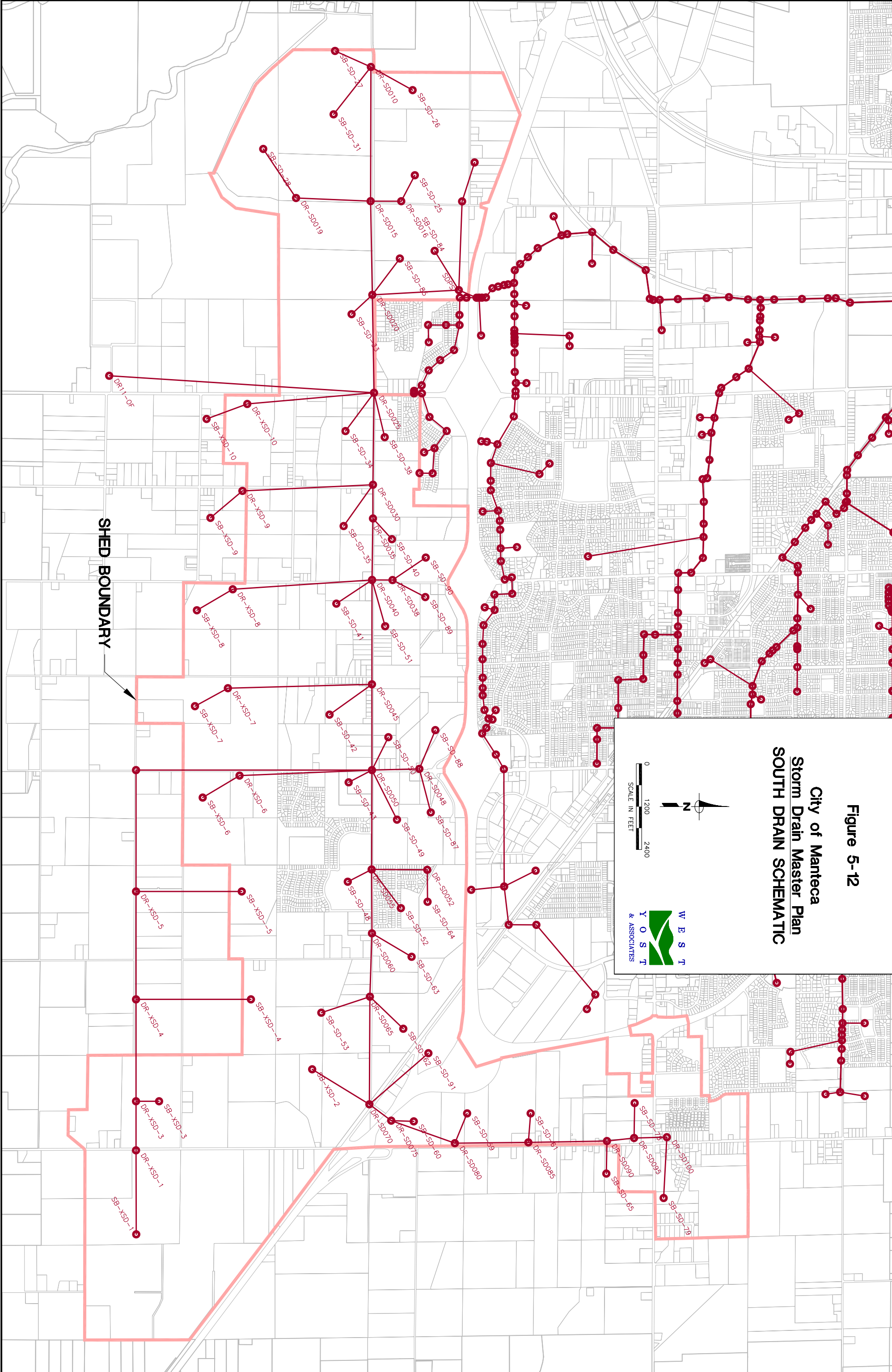
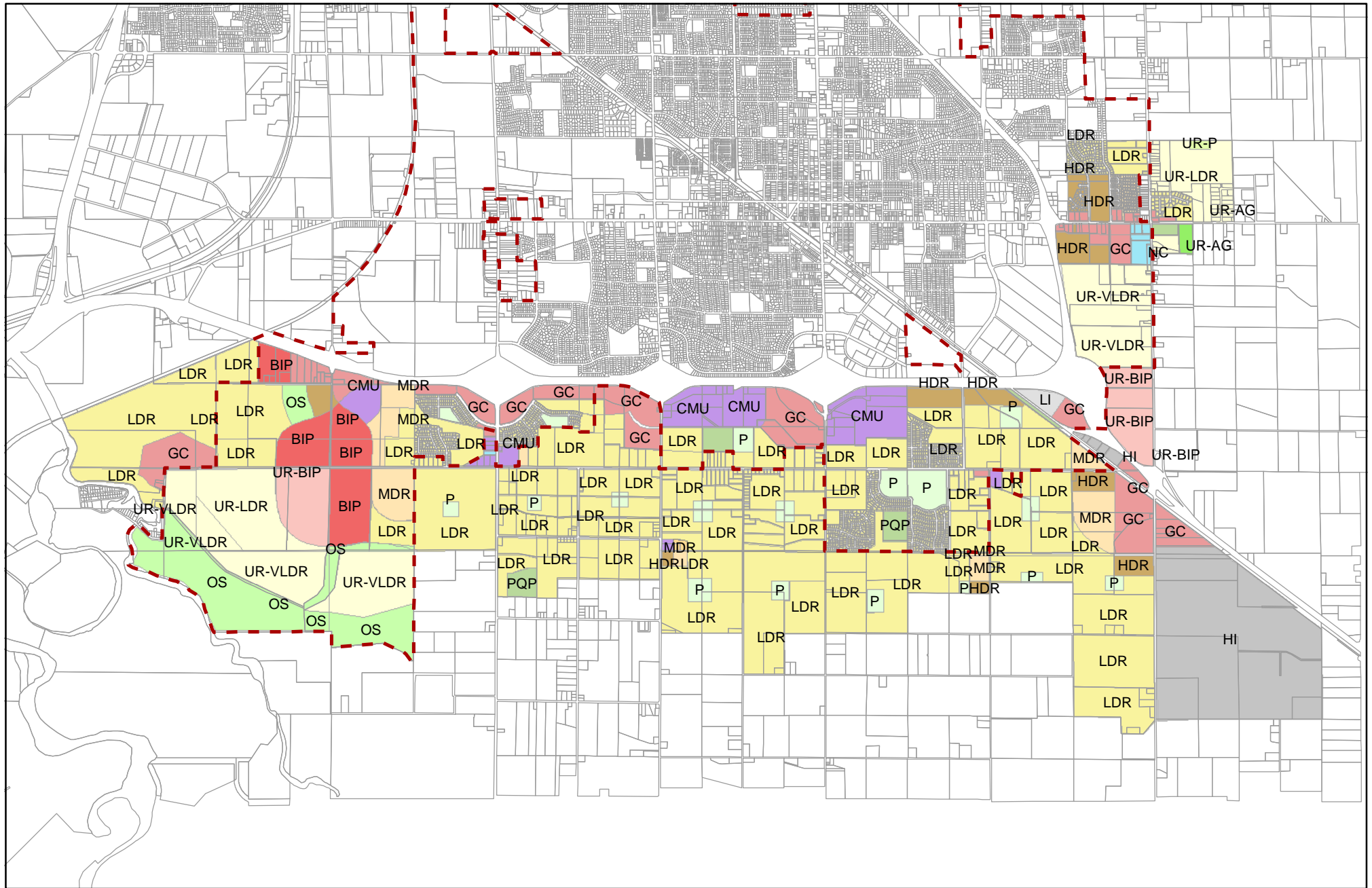


Figure 5-12
City of Manteca
Storm Drain Master Plan
SOUTH DRAIN SCHEMATIC



LAND USE

HDR	MDR	PQP	UR-P
BIP	HI	NC	UR-AG
CMU	LDR	OS	UR-BIP
GC	LI	P	UR-LDR
			VLDR

 City limits

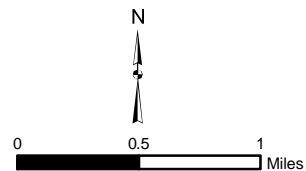


Figure 5-13

City of Manteca
Storm Drain Master Plan
LAND USE - SOUTH DRAIN SUBSHED

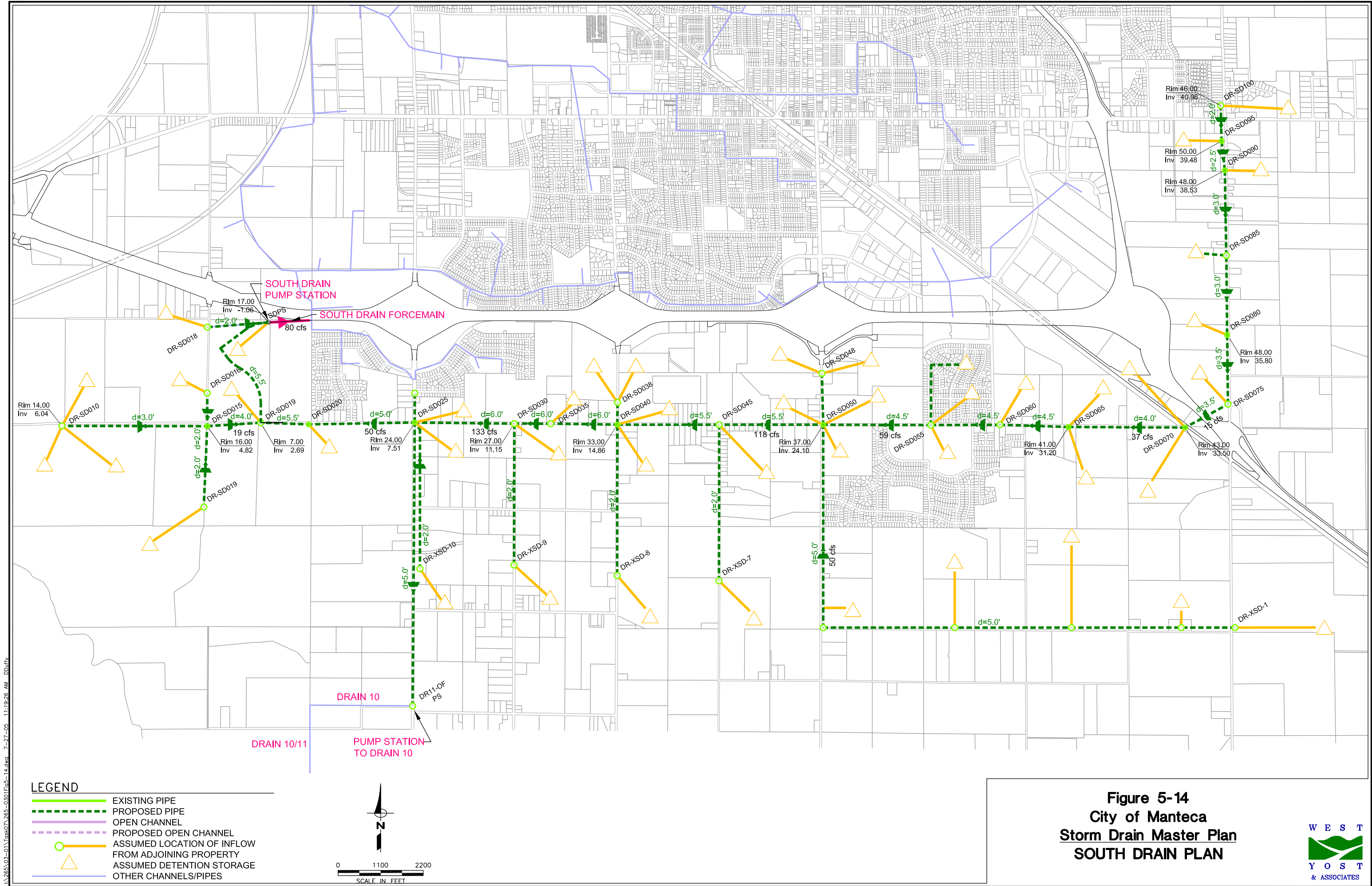


Table 5-14. Proposed South Drain Land Use Data, acres

Subshed Number	VLDR	LDR	MDR	HDR	BIP	CMU	GC	LI	HI	PQP	NC	P	OS	RDS	Total
79	9.90	52.67	0.00	0.00	0.00	0.00	3.30	0.00	0.00	0.00	0.00	0.00	0.00	7.74	73.61
78	0.01	25.48	22.51	38.01	0.00	0.00	26.36	0.00	0.00	0.03	4.78	0.00	0.00	22.02	139.20
65	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.00	0.00	0.00	0.00	2.30	9.30
61	0.00	0.00	0.00	32.96	0.00	0.00	19.79	0.00	0.00	0.00	14.21	0.00	0.00	3.43	70.39
59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	14.74	14.74
60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	7.70	7.70
91	0.00	0.00	0.00	0.00	0.00	0.00	16.64	16.36	0.00	4.53	0.00	0.00	0.00	3.29	40.81
912	0.00	0.00	0.00	0.00	0.00	0.00	8.10	0.00	10.47	0.00	0.00	0.00	0.00	1.51	20.08
53	0.00	152.15	0.00	0.00	0.00	5.01	0.00	0.00	0.00	0.00	0.00	10.24	0.00	0.19	167.59
62	0.00	61.09	14.75	13.40	0.00	0.00	0.00	0.00	0.00	0.65	0.00	5.40	0.00	4.28	99.57
63	0.00	71.27	0.00	6.94	0.00	0.01	0.00	0.00	0.00	0.00	0.00	5.10	0.00	2.31	85.62
64	0.00	21.98	0.00	25.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.81	49.55
48	0.00	131.01	0.00	0.00	0.00	0.01	2.33	0.00	0.00	0.00	0.00	50.62	0.00	25.13	209.10
52	0.00	45.44	0.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	16.62	62.08
87	0.00	0.02	0.00	0.01	0.00	95.88	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.89	96.80
88	0.00	0.00	0.00	0.00	0.00	1.94	71.28	0.00	0.00	0.00	0.00	0.00	0.00	3.25	76.48
43	0.00	127.91	0.00	0.00	0.00	0.00	0.00	0.00	0.00	17.64	0.00	0.00	0.00	9.49	155.05
49	0.00	49.38	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5.00	54.38
50	0.00	48.91	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	12.07	0.00	5.08	66.05
89	0.00	22.44	0.00	0.00	0.00	91.22	0.00	0.00	0.00	36.10	0.00	0.08	0.00	2.80	152.63
90	0.00	22.57	0.00	0.00	0.00	0.00	61.33	0.00	0.00	0.00	0.00	0.00	0.00	2.44	86.35
41	0.00	147.75	3.68	0.06	0.00	3.35	0.00	0.00	0.00	0.00	0.00	9.24	0.00	2.48	166.55
51	0.00	27.17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.02	0.00	0.00	0.00	3.98	31.17
83	0.00	17.07	0.01	0.00	0.00	0.03	27.77	0.00	0.00	0.00	0.00	5.02	0.00	10.73	60.61
86	0.00	34.59	0.00	0.00	0.00	0.00	36.31	0.00	0.00	0.00	0.00	6.95	0.00	11.67	89.51
34	0.00	79.69	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.75	0.00	2.66	87.10
35	0.00	92.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5.74	0.00	2.21	99.97
38	0.00	75.59	5.09	0.00	0.00	9.38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	11.93	102.00
28	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	16.76	0.21	16.97
30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	81.04	0.29	81.32
33	0.00	152.06	0.24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	5.66	0.00	3.20	161.16
18	0.00	36.10	0.00	0.00	27.34	0.00	21.43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	84.86
25	0.00	88.80	0.00	15.21	21.77	0.00	0.00	0.00	0.00	0.00	0.00	0.00	21.85	3.10	150.73
84	0.00	11.81	36.77	2.02	24.35	24.74	18.91	0.00	0.00	0.00	0.00	0.00	0.00	0.38	118.97
27	0.00	0.06	0.00	0.00	0.17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	4.90	5.13
85	0.00	19.97	8.42	0.00	28.21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.03	56.63
26	0.00	40.22	0.00	0.00	35.48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	75.71
24	0.00	232.56	0.00	0.00	0.00	0.00	49.64	0.00	0.00	0.00	0.00	0.00	0.00	0.16	282.35
29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	193.15	7.26	200.41
42	0.00	155.96	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	8.69	0.00	5.84	170.49
39	0.00	65.50	0.00	0.00	0.00	9.96	1.89	0.00	0.00	0.00	2.72	0.00	0.00	7.41	87.49
40	0.00	25.83	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.62	28.45
31	0.00	28.30	48.96	0.13	73.76	0.00	0.00	0.00	0.00	0.00	0.00	0.00	14.57	3.34	169.06
23	0.00	35.14	0.00	0.00	0.00	0.00	9.52	0.00	0.00	0.00	0.00	0.00	0.00	3.57	48.23
Total	9.92	2198.52	140.47	134.48	211.07	241.52	374.57	16.36	10.47	65.98	21.72	129.53	327.36	229.97	

Table 5-15. South Drain

Link Name	u/s Node	d/s Node	GS Elev u/s	GS Elev d/s	US Invert	DS Invert	Length, feet	Slope	Diameter	Max Flow, cfs	Qfull 0.015	Cover	HGL u/s	HGL d/s	Location/Reach
DRPSD095.1	DR-SD100	DR-SD095	46	50	40.96	39.98	750	0.00130	2.0	2.1	7.1	3.0	41.7	41.3	Along Austin Road
DRPSD090.1	DR-SD095	DR-SD090	50	48	39.48	39.03	700	0.00065	2.5	8.9	9.1	8.0	41.3	40.5	Along Austin Road
DRPSD085	DR-SD090	DR-SD085	48	45	38.53	37.40	2250	0.00050	3.0	9.5	13.0	6.5	40.5	39.8	Along Austin Road
DRPSR080	DR-SD085	DR-SD080	45	48	37.40	36.30	2000	0.00055	3.0	13.5	13.6	4.6	39.8	38.1	Along Austin Road
DRPSD075.1	DR-SD080	DR-SD075	48	45	35.80	34.43	2750	0.00050	3.5	14.5	19.6	8.7	38.1	37.0	Along Austin Road
DRPSD070	DR-SD075	DR-SD070	45	43	34.43	34.00	850	0.00050	3.5	15.0	19.6	7.1	37.0	36.7	Along Austin Road
DRPSD065	DR-SD070	DR-SD065	43	41	33.50	31.70	3000	0.00060	4.0	29.9	30.6	5.5	36.7	34.6	Rail Rd to Main Street
DRPSD060	DR-SD065	DR-SD060	41	42	31.20	30.30	1800	0.00050	4.5	36.6	38.3	5.3	34.6	33.5	Rail Rd to Main Street
DRPSD055	DR-SD060	DR-SD055	42	40	30.30	29.15	1750	0.00066	4.5	38.7	43.9	7.2	33.5	32.2	Rail Rd to Main Street
DRPSD050	DR-SD055	DR-SD050	40	37	29.15	25.10	2700	0.00150	4.5	48.1	66.3	6.4	32.2	28.6	Rail Rd to Main Street
DRPSD045	DR-SD050	DR-SD045	37	30	24.10	19.73	2650	0.00165	5.5	117.6	118.9	7.4	28.6	24.2	Main Street to Union Rd
DRPSD040	DR-SD045	DR-SD040	30	33	19.73	15.36	2650	0.00165	5.5	117.6	118.9	4.8	24.2	19.6	Main Street to Union Rd
DRPSD035	DR-SD040	DR-SD035	33	27	14.86	12.41	1750	0.00140	6.0	131.5	138.1	12.1	19.6	17.2	Union Rd to Airport Way
DRPSD030	DR-SD035	DR-SD030	27	27	12.41	11.15	900	0.00140	6.0	132.2	138.1	8.6	17.2	16.1	Union Rd to Airport Way
DRPSD025	DR-SD030	DR-SD025	27	24	11.15	7.51	2600	0.00140	6.0	132.1	138.1	9.9	16.1	11.2	Union Rd to Airport Way
DRPSD020	DR-SD025	DR-SD020	24	17	8.51	4.59	2800	0.00140	5.0	48.1	84.9	10.5	11.2	8.1	West of Airport Way
DRPSD018	DR-SD020	DR-SD019	17	17	4.09	2.69	1400	0.00100	5.5	48.1	92.5	7.4	8.1	7.2	West of Airport Way
DRPSDPS	DR-SD019	DR-SDPS	17	17	2.69	-1.06	3740	0.00100	5.5	50.8	92.5	8.8	7.2	3.6	Airport Way to Drain 8
DRPSD015W.1	DR-SD010	DR-SD015	14	16	6.04	5.32	1600	0.00045	3.5	11.6	18.6	4.5	10.1	9.3	
DRPSD015.1	DR-SD015	DR-SD019	16	17	4.82	4.19	1400	0.00045	4.0	16.7	26.5	7.2	9.3	8.1	
DRPSD017	DR-SD018	DR-SDPS	15	17	8.39	5.59	2800	0.00100	2.0	4.1	6.2	4.6	9.9	3.6	

- A 60 inch diameter south diversion that would convey the remaining portion of South Drain flow south along Airport Way to a pump station that will lift flow into Drain 10 and downstream into Drain 11.

The present South Drain plan includes two drains serving the area east of South Main Street, a Woodward Avenue drain from Austin Road to South Main Street and an industrial lateral from the planned industrial area south of Woodward Avenue then west to South Main Street and north to Woodward Avenue (see Figure 5-14). From Main Street, the South Drain continues west to Airport Way and the pump stations.

As was discussed above, the present General Plan includes changes in land uses and adds new land areas to the South Drain service area; areas that were not included in the 1990 General Plan used for the 1993 study. In the 1993 study, the previous basis for South Drain planning, the South Area produced a peak 10-year outflow after detention of 138 cfs; the present analysis results in an equivalent flow of approximately 166 cfs.

South Drain flows are shown on Table 5-16. Table 5-17, South Drain Water Elevations, shows model output data at each model node.

Western Terminus of South Drain

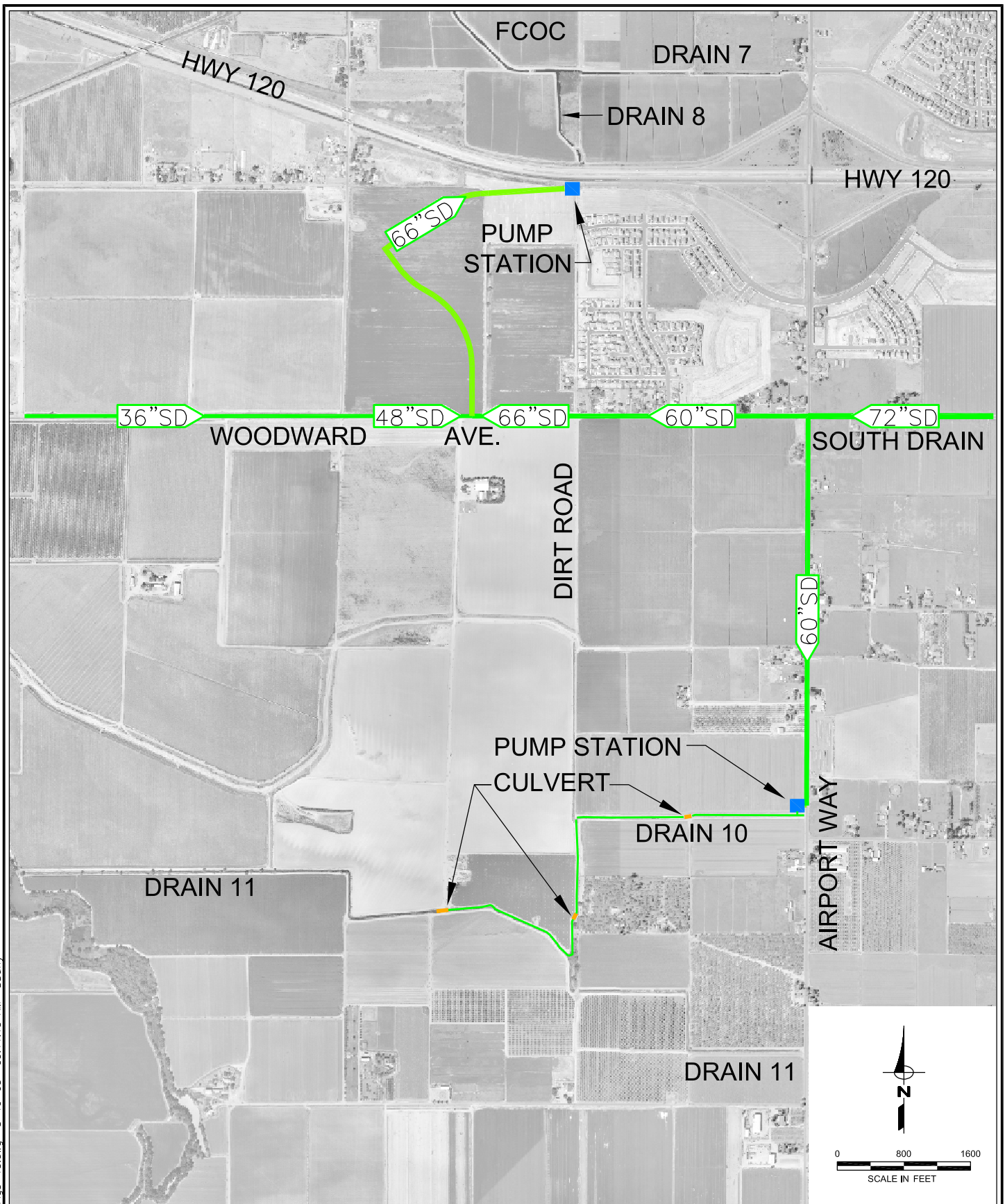
Several potential locations for the western terminus of the South Drain were investigated. These included alternative alignments, pumping facilities and the location of the ultimate receiving waters. A technical memorandum submitted and discussed during the preparation of the Master Plan identified Drain 11 and the FCOC as the optimum South Drain receiving waters. The Drain 11 alignment has some potential problems and a risk of unknowns that may delay construction of the needed facilities. The FCOC is expected to have fewer problems and can be on-line in a shorter time.

A hybrid plan for the ultimate discharge of the South Drain was adopted for the Master Plan. The plan is to use the FCOC alternative for the first several years of the South Area buildout. A second phase would divert additional flows to Drain 11. This hybrid alternative will provide time for the City to pursue the approvals and permits necessary to utilize Drain 11.

An earlier discussion presented the results of the Master Plan analysis of the FCOC and recommended improvements to assure that the facility will meet the needs of the Master Plan buildout period. With completion of the recommended improvements, the Master Plan analysis shows that the FCOC will have capacity to receive about 80 cfs of South Drain flow. Analysis shows that if more than 80 cfs is added from the South Drain, the FCOC will have to be widened at several locations.

The Master Plan provides for two South Drain outfall locations, Figure 5-15. The first 80 cfs will be pumped to Drain 8 and the FCOC and subsequent flows will be directed to and pumped into Drains 10 and 11. The first phase is expected to meet the drainage needs for ten years of South Area development. This estimate is based on present planning and may change with changing conditions and development plans. Planning should begin immediately to assure that the second phase, South Drain outfall to Drain 11, will be in place when South Area development requires.

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LEGEND:

- STORM DRAIN
- NEW CULVERT
- 60"SD — CHANNEL IMPROVEMENTS
- PUMP STATION SITE

Figure 5-15

**City of Manteca
Storm Drain Master Plan
SOUTH DRAIN**



Table 5-16. South Drain - Flows

Upstream Node	Downstream Node	Conduit Class	Conduit Type	Max Flow, ft ³ /s, m ³ /s	Upstream Max Water Elevation, feet, meters	Pipe Diameter/ Box Culvert Width, feet	Box Culvert Height, feet
DR-SD018	SDPS	Circular	PIPE	4.1	9.9	2.0	
DR-SD100	DR-SD095	Circular	PIPE	2.1	41.7	2.0	
DR-SD095	DR-SD090	Circular	PIPE	8.9	41.3	2.5	
DR-SD090	DR-SD085	Circular	PIPE	9.5	40.5	3.0	
DR-SD085	DR-SD080	Circular	PIPE	13.5	39.8	3.0	
DR-SD080	DR-SD075	Circular	PIPE	14.5	38.1	3.5	
DR-SD075	DR-SD070	Circular	PIPE	15.0	37.0	3.5	
DR-SD070	DR-SD065	Circular	PIPE	29.9	36.7	4.0	
DR-SD065	DR-SD060	Circular	PIPE	36.6	34.6	4.5	
DR-SD060	DR-SD055	Circular	PIPE	38.7	33.5	4.5	
DR-SD052	DR-SD055	Circular	PIPE	1.8	36.3	2.0	
DR-SD055	DR-SD050	Circular	PIPE	48.2	32.2	4.5	
DR-SD048	DR-SD050	Circular	PIPE	11.6	32.1	2.5	
DR-XSD-6	DR-SD050	Circular	PIPE	52.5	31.5	5.0	
DR-SD050	DR-SD045	Circular	PIPE	117.6	28.6	5.5	
DR-XSD-7	DR-SD045	Circular	PIPE	4.4	26.0	2.0	
DR-SD045	DR-SD040	Circular	PIPE	117.6	24.2	5.5	
DR-SD038	DR-SD040	Circular	PIPE	14.0	27.1	2.5	
DR-XSD-8	DR-SD040	Circular	PIPE	3.5	23.3	2.0	
DR-SD040	DR-SD035	Circular	PIPE	131.5	19.6	6.0	
DR-SD035	DR-SD030	Circular	PIPE	132.2	17.2	6.0	
DR-XSD-9	DR-SD030	Circular	PIPE	2.6	16.7	2.0	
DR-SD030	DR-SD025	Circular	PIPE	132.1	16.1	6.0	
DR-XSD-10	DR-SD025	Circular	PIPE	3.3	15.2	2.0	
DR-SD025	DR-SD020	Circular	PIPE	48.1	11.2	5.0	
DR-SD016	DR-SD015	Circular	PIPE	4.9	9.5	3.5	
DR-SD019	DR-SD015	Circular	PIPE	0.4	11.9	2.0	
DR-SD010	DR-SD015	Circular	PIPE	11.6	10.1	3.0	
DR-SD015	DR-SD020	Circular	PIPE	16.7	9.3	4.0	
DR-SD020	SDPS	Circular	PIPE	70.1	8.1	5.5	

Table 5-17. South Drain - Water Surface Elevations

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev
DR-SD100	46	40.96	41.7	45.7	-4.0	OK
SB-SD-78	50	40.4	43.7	49.7	-6.0	OK
DR-SD095	50	39.48	41.3	49.7	-8.4	OK
SB-SD-65	48	40.03	43.0	47.7	-4.7	OK
DR-SD090	48	38.53	40.5	47.7	-7.2	OK
SB-SD-61	45	37.66	40.7	44.7	-4.0	OK
DR-SD085	45	37.4	39.8	44.7	-4.9	OK
SB-SD-59	48	36.75	40.6	47.7	-7.1	OK
DR-SD080	48	35.8	38.1	47.7	-9.6	OK
SB-SD-60	45	34.08	37.8	44.7	-6.9	OK
DR-SD075	45	34.43	37.0	44.7	-7.7	OK
SB-SD-91	43	33.63	36.9	42.7	-5.8	OK
SB-XSD-1	43	33.63	38.6	42.7	-4.1	OK
DR-XSD-1	44	34.2	37.5	43.7	-6.2	OK
SB-XSD-2	43	33.63	36.9	42.7	-5.8	OK
SB-XSD-3	43	33.63	35.8	42.7	-6.9	OK
DR-XSD-3	43	33.39	36.8	42.7	-5.9	OK
DR-SD070	43	33.5	36.7	42.7	-6.0	OK
SB-SD-53	41	31.28	33.8	40.7	-6.9	OK
SB-SD-62	41	31.28	34.3	40.7	-6.4	OK
SB-XSD--4	41	31.28	33.5	40.7	-7.2	OK
DR-XSD-4	41	31.59	35.1	40.7	-5.6	OK
DR-SD065	41	31.2	34.6	40.7	-6.1	OK
SB-SD-63	42	30.3	33.3	41.7	-8.4	OK
DR-SD060	42	30.3	33.5	41.7	-8.2	OK
SB-SD-64	40	35.33	36.6	39.7	-3.1	OK
DR-SD052	40	35.33	36.3	39.7	-3.4	OK
SB-SD-52	40	29.15	32.2	39.7	-7.5	OK
SB-SD-48	40	29.15	32.2	39.7	-7.5	OK
SB-XSD--5	40	29.15	32.2	39.7	-7.5	OK
DR-XSD-5	40	29.46	33.2	39.7	-6.5	OK
DR-SD055	40	29.15	32.2	39.7	-7.5	OK
SB-SD-87	35	29.79	31.3	34.7	-3.4	OK
SB-SD-88	35	29.79	31.3	34.7	-3.4	OK
DR-SD048	35	29.79	32.1	34.7	-2.6	OK
SB-SD-43	37	25.84	28.8	36.7	-7.9	OK
SB-SD-49	37	25.84	28.8	36.7	-7.9	OK
SB-SD-50	37	25.84	28.8	36.7	-7.9	OK
SB-XSD-6	37	25.84	28.8	36.7	-7.9	OK
DR-XSD-6	37	27.6	31.5	36.7	-5.2	OK
DR-SD050	37	24.1	28.6	36.7	-8.1	OK
SB-SD-42	30	21.58	23.2	29.7	-6.5	OK
SB-XSD-7	30	21.58	23.4	29.7	-6.3	OK
DR-XSD-7	30	21.78	26.0	29.7	-3.7	OK
DR-SD045	30	19.73	24.2	29.7	-5.5	OK
SB-SD-89	30	24.79	26.3	29.7	-3.4	OK
SB-SD-90	30	24.79	26.2	29.7	-3.5	OK

Table 5-17. South Drain - Water Surface Elevations, cont'd...

Node Name	Model Gr Elev, feet	Inv Elev, feet	10-yr Max WSEL	Flood Elev, feet	Flood Depth, feet	HGL criteria - 0.5 feet below Rim Elev
DR-SD038	30	24.79	27.1	29.7	-2.6	OK
SB-SD-41	33	18.6	21.6	32.7	-11.1	OK
SB-SD-51	33	18.6	21.8	32.7	-10.9	OK
SB-XSD-8	33	18.6	21.6	32.7	-11.1	OK
DR-XSD-8	33	18.8	23.3	32.7	-9.5	OK
DR-SD040	33	14.86	19.6	32.7	-13.1	OK
SB-SD-40	27	15.72	18.7	26.7	-8.0	OK
DR-SD035	27	12.41	17.2	26.7	-9.5	OK
SB-SD-35	27	13.72	16.7	26.7	-10.0	OK
SB-XSD-9	27	13.72	16.7	26.7	-10.0	OK
DR-XSD-9	27	13.92	16.7	26.7	-10.0	OK
DR-SD030	27	11.15	16.1	26.7	-10.6	OK
SB-SD-34	24	10.96	14.0	23.7	-9.7	OK
SB-SD-38	24	10.96	14.2	23.7	-9.5	OK
SB-XSD-10	24	10.96	14.0	23.7	-9.7	OK
DR-XSD-10	24	11.16	15.2	23.7	-8.5	OK
DR-SD025	24	7.51	11.2	23.7	-12.5	OK
SB-SD-33	17	7.28	9.8	16.7	-6.9	OK
DR-SD020	17	4.09	8.1	16.7	-8.6	OK
SB-SD-28	25	19.79	20.8	24.7	-3.9	OK
DR-SD019	25	11.55	11.9	24.7	-12.8	OK
SB-SD-85	16	3.06	6.9	15.7	-8.8	OK
DR-SD015	16	6.85	9.3	15.7	-6.4	OK
SB-SD-25	15	8.01	10.3	14.7	-4.4	OK
SB-SD-84	15	8.01	11.0	14.7	-3.7	OK
DR-SD016	15	8.2	9.5	14.7	-5.2	OK
SB-SD-18	15	10.33	11.5	14.7	-3.2	OK
SB-SD-26	14	0.98	4.7	13.7	-9.0	OK
SB-SD-27	14	0.98	5.4	13.7	-8.3	OK
SB-SD-31	14	0.98	4.9	13.7	-8.8	OK

South Drain Flows - First Phase - FCOC

The FCOC is a SSJID facility that conveys agricultural and urban drainage flows north from the vicinity of Highway 120 to French Camp Road and its outfall to French Camp Slough. The SSJID allows the City to discharge stormwater into the FCOC. Restrictions on the drainage flows that the District will accept in the FCOC and a growing Manteca have prompted the City to consider the need to expand the FCOC.

The Master Plan analysis using the City's XP-SWMM model demonstrates that 80 cfs of South Drain flow can be pumped into Drain 8 at Highway 120. This 80 cfs is about 48 percent of the ultimate South Area flow of 166 cfs and should provide capacity until about 2015. This South Drain flow will combine with Drain 8 flow and then combine with Drain 7 at the beginning of the FCOC.

Directing first phase South Drain flows to the FCOC requires that Master Plan improvements to Drain 8 and the FCOC be completed including construction of the following facilities:

1. Construction of FCOC Master Plan improvements
2. Construction of Drain 8 Master Plan improvements
3. Continuation of the South Drain from Woodward Avenue north to Highway 120
4. Construction of a pumping plant at Highway 120 to lift South Drain flows into Drain 8

South Drain from Airport Way to Highway 120

The first phase extension of the South Drain from Airport Way west on Woodward Avenue and north to the pump station (see Figure 5-15) to the pumping plant at Highway 120 will be a 60-inch diameter pipe, 5,200 feet in length. For flexibility and reliability, the pump station will have four 27 cfs units, one of which will be a standby unit. The pump discharge lines will manifold to a 30-inch pressure pipe jacked under the highway to Drain 8.

South Drain Flows Second Phase - Drain 11

The second phase South Drain outfall proposes to use SSJID Drains 10 and 11 to carry a portion of South Drain flow to an existing outfall at the San Joaquin River, Figures 5-15. The South Drain would be extended south along Airport Boulevard. A 60-inch diameter pipe would carry 90 cfs from Woodward Avenue south for 4,800 feet to Drain 10. A pump station constructed at the levee would lift drainage flows to the Drain 10 channel and conveyed 4,480 feet west to its confluence with Drain 11.

Drain 10 would be enlarged from Airport Way to its confluence with Drain 11. Drain 11 would be enlarged to convey the South Drain flow 7,150 feet west to an existing outfall at Walthall Slough. Four culvert structures would be re-constructed under access roads. A Drain 11 culvert at the Walthall Slough levee and an upgraded outfall structure at the slough would also be required.

The Airport Way / Drain 10 pump station would have 4 units totaling 90 cfs including a standby pump and motor unit. Pumps will lift South Drain flow about 11 feet from the South Drain pipe in Airport Way to the new channel.

The SSJID's Drains 10 and 11 will be widened from channel widths of 27 and 47 feet to 32 and 50 feet respectively. Adequate right of way will be obtained to provide for maintenance activities and access.

If Drains 10 and 11 are to be part of the south area drainage plan, agreements will have to be worked out between the SSJID and the City. There will have to be assurances that operations of the District are not compromised by conveying the City's drainage and that pumps discharging into the South Drain will be cycled or shut down to maintain desired water surface elevations in Drain 11. Use of the District facilities would be governed by the same restrictions that regulate the City's use of other SSJID drains. These requirements will necessitate the construction of adequately sized detention basins and similar telemetry controlled discharges that are in place for other SSJID drains used by the City.

Environmental issues include concerns over the widening of existing channels, re-constructing culverts at road crossings, excavating through the Walthall Slough levee to construct a new culvert and constructing of a larger outfall structure in Walthall Slough. The wider channels will provide opportunities for additional wetland habitat, and there will be opportunities for new habitat restoration along Drain 11. Widening the drains to carry higher flows will require mitigation to alleviate potential concerns of landowners along the channels. The impact of using the existing outfall location on Walthall Slough is somewhat lessened due to its historical use for discharge of runoff from the tributary area.

Master Plan Projects

The recommended improvements for the proposed South Drain are summarized below

Location	Recommended Improvements
• South Drain	Construct South Drain to Airport Way
• From Airport Way along Woodward Avenue and north to Highway 120	Construct 60-inch pipe, 5,200 feet
• Pump Station at Hwy 120	
• Airport Way to Drain 10	Construct 60-inch pipe, 4,800 feet
• Pump Station at Drain 10	
• Drain 10 and 11	Widen channels

SSJID LATERALS

Some of the SSJID irrigation supply laterals are used by the City to convey drainage. The Master Plan provides for the continued use of the lateral where they are providing a service. Since they are an integral part of the City drainage system the laterals used for drainage are described below.

The SSJID laterals are the delivery pipes and channels distributing irrigation water to fields throughout the District. Manteca pumps drainage runoff to six SSJID laterals. Each of these laterals discharge to a drain or directly to the FCOC. Laterals are designed to be higher to allow gravity delivery of water to fields. Because their function is delivery and not drainage, it is

necessary to limit City drainage flows into laterals. SSJID limits flow in its laterals from City drains to 25 cfs.

Laterals Rf, Y and Z are affected by Master Plan projects. Drain 3A, which is now pumped into Lateral Rf at Pump Station 15 will be directed into Drain 3N. This was the only City drainage pumped into the lateral. Pump Station 8 used to pump to Lateral Y and Pump Station 9 used to pump to Lateral Z. Now both pump to Drain 7. Pump Station 17 continues to pump drainage to Lateral Z and Pump Station 7 continues to pump drainage to Lateral Y.

Lateral Re - receives about 7.7 cfs from Pump Stations 3 and 4 and discharges into Drain 3.

Lateral T - receives drainage from Pump Stations 1, 2, 10, 14, A, B, C, D and E. With a total drainage flow of 30 cfs, Lateral T is above the 25 cfs limit. Lateral T flows directly into the FCOC.

Lateral Tb - receives inflow from Pump Station 13. Lateral Tb terminates in Drain 5.

Lateral Z - receives inflow from Pump Stations 17 and 24 and discharges into Lateral Y.

Lateral Y - receives inflow from Pump Station 7 and from Lateral Z. Lateral Y is being relocated along Stadium Drive and discharges into Drain 7.

DETENTION BASINS

The Manteca Storm Drain Master Plan requires that all runoff collected in storm drains flow to a detention basin before being pumped into one of the drains that will convey flow to the FCOC and the Delta. Basins have traditionally been used to attenuate peak flows prior to entering drains to meet restrictions of the SSJID. In the future, most new basins will have a dual purpose to also provide stormwater treatment and reduce the pollutants in stormwater entering the drains.

Basins are sized in accordance with City design criteria, Chapter 3. The basic criterion is that basins are sized for the watershed runoff with 3.56 inches of rainfall. This storage volume was used in the XP-SWMM analysis for existing, proposed and future Master Plan basins. The modeling showed that even with some basins holding their water for a longer time, the storage volume appeared to be adequate. The City's model should be a valuable tool to evaluate future proposals.

Water quality treatment options are included in the design criteria of Chapter 3 and in Chapter 6, Water Quality. Water quality complicates multi-use of basins, at least for part of the basin area, but each situation should be analyzed in detail to obtain the maximum benefits of a joint use facility. Preliminary designs should also be reviewed by Parks and Recreation for their assistance and approval.

Table 5-18 shows existing basins including those under construction. In the coming years, a significant number of new basins will be required as shown in Table 5-19. Any efforts that landowners and developers may make to combine proposed developments and construct regional facilities would lead to operational difficulties. There is no land use policy in place that allows the City to direct the construction of regional detention basins. For the new developing land areas, sub-sheds were delineated and basins were sized and modeled.

Table 5-18. Existing Detention Basins

Name	Location	Storm Drainage Volume, ac-ft	Basin Outlet	Controlled Outlet
Shasta Park	Place Ave and Snowshoe Way	15.9	Drain 3	Pump Station #19
Colony Park	Trailwood Avenue and Cedar Way	6.4	Drain 3	None
Springtime Park	April Way and Springtime Avenue	4.7	Drain 3	None
Doxey Park	Northgate Drive and Doxey Drive	6.1	Drain 3	Pump Station #12
Crestwood Park	Cottonwood Drive and Crestwood Avenue	3.6	Drain 3A	None
Spring Meadows	Pestina Avenue and Orange Blossum Drive	6.3	Drain 3	Proposed Pump Station
Button Estates/Discovery Creek	Discovery Creek	6.8	Drain 3	Pump Station #21
K-Mart	Northgate and Main	2.5	Drain 3	Electric Gate
Cardoza West	Louise and Main	2.0	Drain 3	Electric Gate
Chadwick Square Park	London Avenue and Exeter Drive	17.3	Drain 3	Pump Station #23
Diamond Oaks Park	Pestana Avenue, and Diamond Oaks Way	7.2	Drain 3	Pump Station #22
Cheeks Creek	Along Drain 3 between Trailwood Avenue and Union Road	6.5	Drain 3	None
Bay Meadows Park	Elm Avenue and Alameda Street	5.1	Drain 4	None
Greystone Park	Agate Avenue and Turquoise Way	4.3	Drain 4	Pump Station #11
Mayor's Park	Stoker Way and Phillips Drive	6.0	Drain 4	Pump Station #16
Walnut Place Park	Union Pacific RR and Pioneer Avenue	3.21	Drain 4	Pump Station #25
Primavera Park	Primavera Avenue and Giuciano Way	16.7	Drain 4	Proposed Pump Station
Curran Grove Park	Schadeck Street and Bernt Avenue	12.7	Drain 5	Pump Station #26

Table 5-18. Existing Detention Basins, cont'd...

Name	Location	Storm Drainage Volume, ac-ft	Basin Outlet	Controlled Outlet
Union West Park	Mercedes Avenue and Parkview Street	4.2	Drain 5	None
Villa Tocino	Zurcih Drive and Solothum Eway	13.1	Drain 5	Pump Station #28
San Dominic	St. Dominic's Drive	29.3	Drain 5	None
Quail Ridge Park	Mission Ridge Drive	2.2	Drain 7	Pump Station #5
Vintage Estates/Cotta Park	Mission Ridge Drive	5.5	Drain 7	Pump Station #20
Brock Elliott School	Daniel Street	13.2	Drain 7	None
Roberts Estates Park Basin	Hidden Brook Drive and Junction Drive	13.8	Drain 7	Pump Station #27
Spreckels Baseball Park Basin	Moffat Blvd. And Spreckels Avenue	47.5	Drain 7	
Sequoia Park	Wawona and Ebbetts	2.0	Lateral y	Pump Station #7
Gonsalves Park/Fishback Basin	Fishback Road	6.0	Lateral Z	Pump Station #24
Bella Vista (Proposed)	Proposed Streets	16.64	Drain 8	Proposed Pump Station
Woodward Park	Woodward Avenue and Birdwell Avenue	62.3	Future South Drain	Proposed Pump Station
St. Francis Park	Devonshire Avenue and Pheasant Hallow Way			None

Table 5-19: Proposed Storm Water Pump Stations

Basin ID	Pump Outflow Rate, cfs	Pumping Capacity, gpm	Discharge To	Drainage Area, acres	Detention Basin, acre-feet
SB 7-C	6.99	3,140	Drain 7	110.00	27.74
Industr Pk	15.89	7,135	Drain 7	250.00	63.04
DR7-BLD	4.34	1,947	Drain 7	40.28	10.75
DR-GCSB	2.71	1,217	Drain 7	61.21	17.20
HTL & GS	0.39	174	Drain 7	5.84	1.56
DR-7017	2.30	1,033	Drain 7	34.00	9.10
SB-SD-79	2.15	964	South Drain	73.60	8.52
SB-SD-78	6.70	3,008	South Drain	139.20	26.57
SB-SD-65	0.63	281	South Drain	9.30	2.48
SB-SD-61	4.00	1,795	South Drain	70.40	15.86
SB-SD-59	0.99	446	South Drain	14.70	3.94
SB-SD-60	0.52	233	South Drain	7.70	2.05
SB-SD-91	4.10	1,841	South Drain	60.90	16.26
SB-SD-53	3.92	1,759	South Drain	167.60	15.54
SB-SD-62	2.94	1,318	South Drain	99.60	11.65
SB-SD-63	2.14	962	South Drain	85.60	8.50
SB-SD-64	1.77	795	South Drain	49.50	7.02
SB-SD-48	5.55	2,490	South Drain	209.10	22.00
SB-SD-52	2.14	960	South Drain	62.10	8.48
SB-SD-87	6.51	2,925	South Drain	96.80	25.84
SB-SD-88	5.15	2,311	South Drain	76.50	20.42
SB-SD-43	4.70	2,109	South Drain	155.00	18.63
SB-SD-49	1.44	649	South Drain	54.40	5.73
SB-SD-50	1.62	727	South Drain	66.10	6.42
SB-SD-42	4.02	1,806	South Drain	170.50	15.95
SB-SD-89	9.26	4,159	South Drain	152.60	36.74
SB-SD-90	4.80	2,155	South Drain	86.30	19.04
SB-SD-41	3.99	1,790	South Drain	166.60	15.81
SB-SD-51	0.88	395	South Drain	31.20	3.49
SB-SD-83	3.05	1,370	Drain 8	60.60	12.10
SB-SD-86	4.11	1,845	Drain 8	89.50	16.30
SB-SD-34	2.04	915	South Drain	87.10	8.09
SB-SD-35	2.30	1,032	South Drain	100.00	9.12
SB-SD-38	3.32	1,491	South Drain	102.00	13.17
SB-SD-28	0.26	119	South Drain	17.00	1.05
SB-SD-30	1.23	553	Drain 10/11	81.30	4.88
SB-SD-33	3.72	1,671	South Drain	161.20	14.76
SB-SD-18	4.09	1,837	South Drain	84.90	16.23
SB-SD-25	4.68	2,099	South Drain	150.70	18.55
SB-SD-84	6.33	2,843	South Drain	119.00	25.12
SB-SD-27	0.34	154	South Drain	5.10	1.36
SB-SD-85	2.66	1,196	South Drain	56.60	10.57
SB-SD-26	3.29	1,478	South Drain	75.70	13.05
SB-SD-24	8.57	3,848	Drain 10/11	282.40	33.99
SB-SD-29	3.38	1,517	Drain 10/11	200.40	13.40
SB-SD-39	2.95	1,324	Drain 8	87.50	11.70
SB-SD-40	0.76	339	South Drain	28.50	3.00
SB-SD-31	7.88	3,537	South Drain	169.10	31.25
SB-SD-23	1.67	750	Drain 10/11	48.20	6.62
SB-XSD-1	38.05	17,087	South Drain	565.40	150.96
SB-XSD-2	10.76	4,830	South Drain	198.40	42.67
SB-XSD-3	7.14	3,205	South Drain	318.20	28.32
SB-XSD--4	1.27	569	South Drain	56.50	5.03
SB-XSD--5	2.37	1,064	South Drain	79.60	9.40
SB-XSD-6	4.37	1,961	South Drain	154.50	17.33

Table 5-19: Proposed Storm Water Pump Stations cont'd...

Basin ID	Pump Outflow Rate, cfs	Pumping Capacity, gpm	Discharge To	Drainage Area, acres	Detention Basin, acre-feet
SB-XSD-7	4.42	1,983	South Drain	196.90	17.52
SB-XSD-8	3.50	1,573	South Drain	156.20	13.90
SB-XSD-9	2.54	1,138	South Drain	113.00	10.06
SB-XSD-10	3.34	1,501	South Drain	149.00	13.26
SB-5-GG	9.97	4,474	Drain 5	281.90	39.53
SB-5-II	3.80	1,707	Drain 5	119.77	15.08
SB-5-JJ	4.73	2,124	Drain 5	148.92	18.76
XDR3-A	24.30	10,910	Drain 3	361.00	96.39
XDR3-B	11.86	5,325	Drain 3	529.00	47.08
XDR3-C	1.40	630	Drain 3	62.50	5.56
SB-F-LDRES	4.37	1,962	Drain 3	194.30	16.94
SB-3-A	2.89	1,299	Drain 3	118.77	11.47
SB-3-B	2.12	951	Drain 3	81.60	8.40
SB-3-D	3.38	1,517	Drain 3	86.87	13.40
SB-3-E	0.91	409	Drain 3	31.84	3.63
SB-3-F	4.48	2,012	Drain 3	170.57	17.77
SB-3N-H	6.53	2,932	Drain 3	197.35	25.91
SB-3N-A	3.81	1,710	Drain 3	161.74	15.10
SB-3N-B	3.63	1,628	Drain 3	159.35	14.38
SB-3N-C	3.85	1,729	Drain 3	160.17	15.27
SB-3N-D	3.68	1,652	Drain 3	160.74	14.60
SB-3N-E	1.85	829	Drain 3	80.53	7.33
SB-3N-F	7.82	3,510	Drain 3	166.12	31.01
SB-3N-G	10.96	4,922	Drain 3	164.20	43.48
SB-3A-A	3.66	1,642	Drain 3	139.69	14.50
SB-3A-B	5.87	2,637	Drain 3	147.04	23.30
3N-CC-1	4.09	1,836	Drain 3	118.00	16.23
3N-CC-2	2.73	1,226	Drain 3	80.00	10.82
DR-Un Ranch	2.25	1,010	Drain 3	78.00	7.60
F-FC-LI	25.61	11,498	FCOC	380.48	101.59
F-FC-COMM	5.29	2,377	FCOC	78.66	21.00
Shed -Z	12.54	5,632	FCOC	261.50	49.76
F-HWYPQP	16.78	7,534	FCOC	249.32	66.57
F-HWYCOMM	3.36	1,506	FCOC	49.85	13.31
South Drain PS	80.00	35,900	Drain 8	—	—
Drain 10 PS	90.00	40,400	Drain 10	—	—

PUMPS

The Manteca drainage system depends on pumping to convey water from storage basins to drains and laterals. Existing pumps were modeled as part of the Master Plan XP-SWMM model from data derived from as-built drawings and site inspections. Table 5-18 lists the existing pump stations.

For each of the future basins identified in Table 5-19, it was assumed that two pumps would be installed, one of which would be standby. Pumps were sized according to City design criteria, Chapter 3. Landowners and the City should make an effort to develop fewer and regional basins. The number of future pump stations in Table 5-19 shows a situation that can only be improved with larger regional basins.